

MOHAMMED MAINUL HOSSAIN

Numerical analysis of vibrations
of nanobeams



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CONTENTS

List of original publications	7
1. Introduction	8
1.1. Literature review	8
1.2. Aim of the dissertation	10
1.3. Structure of the dissertation	11
2. Governing equations and assumptions	12
2.1. Nonlocal theory	12
2.2. Euler-Bernoulli theory	13
2.2.1. Transform functions	13
2.2.2. Non-dimensional parameters	14
2.2.3. Boundary conditions	14
2.3. Modeling of cracks	14
2.4. Tapered beams	16
2.5. Axially graded beams	17
2.6. Thermal loading	17
2.7. Homotopy perturbation method	17
2.8. Maclaurin series	19
2.9. Power series	19
3. The effect of rotatory inertia on the natural frequency of cracked and stepped nanobeams	21
3.1. Formulation of the problem	21
3.2. Mathematical model	22
3.3. Solution	23
3.4. Results and discussion	24
4. Free vibration analysis of the tapered cracked double nanobeams using Maclaurin series	27
4.1. Physical description	27
4.1.1. Governing equations	27
4.2. Solution technique	29
4.2.1. Cracked double nanobeams	32
4.3. Results and discussion	32
4.3.1. Validation and verification	32
4.3.2. Effects of crack locations	33
4.3.3. Mode shape analysis	33
4.3.4. Parametric effects	35

5. Transverse vibration of tapered nanobeams with elastic supports	37
5.1. Problem description	37
5.2. Mathematical model	37
5.2.1. Elastic boundary conditions	38
5.3. Solution of the problem	40
5.4. Results and discussion	41
6. Natural vibration of axially graded multi cracked nanobeams in thermal environment using power series	46
6.1. Description of the problem	46
6.2. Mathematical model	46
6.2.1. Derivation of the governing equation	46
6.3. Power series solution	47
6.4. Numerical results and discussion	48
6.4.1. Comparison of results	49
6.4.2. Effect of crack locations	51
6.4.3. Frequency for axially graded intact and cracked nanobeams under thermal load	51
6.4.4. Mode shape illustration	53
7. Concluding remarks	55
Bibliography	56
Summary	62
Kokkuvõte	63
Acknowledgements	64
Publications	65
The effect of rotatory inertia on natural frequency of nanobeam	69
Transverse vibration of tapered nanobeam with elastic supports	85
Analysis of free vibration of tapered cracked double nanobeams	109
Natural vibration of axially graded multi cracked nanobeam	131
Curriculum Vitae	149
Elulookirjeldus (Curriculum Vitae in Estonian)	151

LIST OF ORIGINAL PUBLICATIONS

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2. Mainul Hossain and Jaan Lellep, Analysis of free vibration of tapered cracked double nanobeams using Maclaurin series, Eng. Res. Express, **4(2)**, 025034 (2022).
3. Mainul Hossain and Jaan Lellep, Transverse vibration of tapered nanobeam with elastic supports, Eng. Res. Express, **3(1)**, 015019 (2021).
4. Mainul Hossain and Jaan Lellep, Natural vibrations of nanostrips with cracks, Acta et Com. Uni. Tartuensis De Math., **25(1)**, 87-105 (2021).
5. Mainul Hossain and Jaan Lellep, Thermo mechanical vibration of single wall carbon nanotube partially embedded into soil medium, Agron. Res., **19(1)**, 777-787 (2021).
6. Mainul Hossain and Jaan Lellep, Mode shape analysis of dynamic behaviour of cracked nanobeam on elastic foundation, Eng. Res. Express, **3(4)**, 045003 (2021).
7. Mainul Hossain and Jaan Lellep, The effect of rotatory inertia on natural frequency of cracked and stepped nanobeam, Eng. Res. Express, **2(3)**, 035009 (2020).
8. Mainul Hossain and Jaan Lellep, Effect of temperature on dynamic behavior of cracked metallic and composite beam, Vibroengineering Procedia, **32**, 172-178 (2020).
9. Jaan Lellep and Mainul Hossain, Free vibrations of rectangular nanoplate strips, The proceedings of the 13th international conference Modern Building Materials, Structures and Techniques, 743-749 (2019).
10. Mainul Hossain and Jaan Lellep, Natural vibration of stepped nanoplate with crack on an elastic foundation, IOP Conf. Ser. Mater. Sci. Eng., **660**, 012051 (2019).

Author's contribution

The author of this dissertation is responsible for majority of the research in all phases (including simulation, analysis, writing and presenting results) of the papers (1-10). The solution techniques were developed in co-operation with the supervisor; the statement of the problem belongs to the supervisor.

1. INTRODUCTION

1.1. Literature review

In recent years, researchers have been attracted by nanomaterials due to their exceptional properties and unavoidable roles in modern science and technology. These exceptional characteristics distinguish nanomaterials from their bulk counterparts. Nanomaterials consist of a larger surface volume ratio than macro-materials. An increase in surface volume ratio increases the number of surface atoms. These surface atoms have unsatisfied bonds that make nanomaterials more reactive than their bulk counterparts (see Li and Wang [45], Li et al. [49]). That is why, nanomaterials possess some strange and vital characteristics such as high strength, high stiffness, and excellent thermal and electrical conductivities (see Abid et al. [1]).

During the application, the nanomaterials are subjected to various forces such as compression, tension, and vibration. In order to overcome the adverse effects of these forces, effective modeling is essential. Several techniques are involved to investigate the behavior of nano-structure such as molecular dynamics simulation, and numerical methods. Among them, the numerical technique is cost-effective, easy to handle, and a widely accepted technique among researchers (Pham et al. [61]). However, the existing continuum theory is not adequate for numerical analysis because it cannot incorporate the small-scale effect (Adhikari et al. [2], Ahmadi [4]). Researchers introduce the gradient theories of continuum mechanics to analyze the nano-devices and to encounter the size effect (Faghidian [21]). Basically, there are two types of gradient theories such as the strain and the stress gradient theories of elasticity that are engaged to analyze nanomaterials (Faghidian [22], Karami and Janghorban [40]). Generally, the stress gradient theory is proposed on the basis of an integro-differential equation whose solution is difficult. That is why Eringen (see Eringen [18]) proposed equivalent differential nonlocal elasticity theory which has been widely accepted to analyze nanomaterials.

Research on nanobeams has started in the recent decade. Researchers have engaged in this research profoundly to explore the behavior of nanobeams. Here, the contributions of some authors studying nanobeams are presented briefly. First of all, the nonlocal Euler–Bernoulli beam was investigated by Eltaher et al. [16] using the finite element method. They presented the efficiency of the model to analyze nanobars, nanotubes, and nanobeams. In another paper, Eltaher et al. [17] described the coupling effects of the nonlocal parameter and surface energy on vibration analysis of nanobeams. They revealed that the surface properties had significant effects on the fundamental frequency in nano and micro-structures. In addition, free vibration of the nonlocal beam was presented by Ke and Wang [41]. They described the effects of electric potential, magnetic potential, and temperature rise. Their results revealed that the electric and magnetic loadings had significant effects on the natural frequency of nanobeams. After that, Simsek [70]

analyzed the free vibration of nanobeams with large amplitude. He examined the effect of the nonlocal parameter on the nonlinear frequency ratio. His model produced a larger nonlinear frequency ratio than the classical (local) beam model.

Ahmadvand and Asadi [5], Chinka et al. [11], Kala [38], Kumar and Singh [42], Rosa and Lippiello [67], Sushobhan and Khazanovich [75] studied the crack as a newly generated surface that partially separated the intact body or structure. During the formation of a crack, the surface area is created in the material in a thermodynamically irreversible manner. The crack is created due to environmental corrosion, operational condition, inappropriate loading, etc. Natural frequency is significantly affected by crack location and cracks severity (Eroglu and Tufekci [19]). The mode shape of the vibration of nanomaterials is significantly changed by the presence of cracks (Hossain and Lellep [34], Song et al. [72], Wu et al. [78]).

After analyzing the nanobeam, several authors showed their interest in cracked nanobeams. First of all, Hashemi et al. [23] explored the dynamic behavior of thin and thick cracked nanobeams with surface effects. Their results illustrated that the surface effects on natural frequencies depended on the length of nanobeams. On the other hand, the effects of cracks on natural vibration did not depend on the length of nanobeams. In addition, the longitudinal vibration of cracked nanobeams was scrutinized by Hsu [37]. This analysis showed that the increase of nonlocal parameters decreased the effect of cracks on the natural frequency. Similarly, Roostai and Haghpanahi [66] considered the vibration of nanobeams with multiple cracks. They observed the effects of crack locations and parameters on the vibration of nanobeams. After that, Hasheminejad et al. [24] explained the dynamic behavior of cracked nanobeams with surface effects. Moreover, Aria et al. [6] demonstrated the vibration of cracked nanobeams on elastic matrix. They observed the effects of crack severity, crack location, and temperature on the natural frequency of nanobeams. Moreover, Bahrami [8] applied a wave-based method to analyze cracked nanobeams. It was shown in [8] that the natural frequency decreased with the increase of the nonlocal parameter. Whereas, in a cantilever beam, the natural frequency increased with the increase of the nonlocal parameter. The free vibration of multi-cracked and stepped nonlocal nanobeams was studied by Loghmani and Yazdi [51]. They observed that the crack had no impact on frequency when it was located at the inflection points of the mode shape.

The advanced manufacturing process overcomes the difficulties to fabricate the non-prismatic beams consisting of complex cross-sections (Ece et al. [15], Yan et al. [79], Zhang et al. [81]). Stepped (see Lellep and Lenbaum [43], Lellep and Lenbaum [44]), tapered, sandwich (see Barkanov [9]) and axially graded beams (Li et al. [48], Mamaghani et al. [55], Rajasekaran and Khaniki [64]) are the forms for optimal design to improve the performance of structures (Karami and Janghorban [40]). Due to the reasonable choice of dimension, the stepped beam is economical because it provides a larger cross-section along the critical segment

instead of all over the span. Stepped beams under vibration load face complex excitation that may be the cause of failure. It is essential to develop a reliable model to study stepped beams under vibration (Afefy et al. [3], Ni and Hua [59], Šalinić et al. [68], Su et al. [73], Surla et al. [74]). In addition, a tapered beam is preferable to the designer because of its cost-effective and material-saving properties (see Chockalingam et al. [12], Chockalingam et al. [13], Prasad and Banerjee [62], Rajasekaran and Khaniki [63], Wan et al. [77]). There is no abrupt change in cross-section or geometric properties as well as there is no stress concentration or fatigue failure. Moreover, an axially graded beam (Datta [14], Li et al. [47], Šalinić et al. [68], Zhang et al. [82]) is a type of composite where the material properties differ smoothly from one end to another end for eliminating the stress concentration at any specific cross-section. The axially functionally graded material is one of the exceptional innovations of recent science and technology that satisfies the growing demand for advanced materials having exceptional properties for high-quality design.

Solution techniques are essential for proper analysis. Some solution techniques have been presented in this study where these techniques are rare in the existing literature to analyze the nano-structures. The homotopy perturbation method, power series, and Maclaurin series are used in this research. First of all, the homotopy perturbation method developed by He (see He [25-27]) is a useful technique for analyzing nonlinear problems. It is the combination of homotopy analysis and perturbation technique where it overcomes the limitations of the conventional perturbation method. In addition, the power series solution technique (see Soltani et al. [71]) is also used for analyzing the differential equations. Basically, the power series is an infinite series. Any polynomial can be expressed as a power series around a certain point. Moreover, the Maclaurin series is a semi-analytical technique based on the Taylor series. Generally, the Maclaurin series is a special form of the Taylor series where the derivatives are calculated at the initial position. All these above techniques have been successfully employed during this analysis.

1.2. Aim of the dissertation

The aim of the dissertation is to analyze the dynamic behavior of nanobeams with different physical and geometrical properties using semi-analytical and numerical techniques. Euler-Bernoulli beam theory and Eringen's nonlocal theory of elasticity are applied to derive the governing equations of nanobeams. Cracks, steps, and taper ratios are considered as geometric irregularities. On the other hand, axially graded material, and surrounding temperature are considered as physical properties.

The significant feature of this analysis is the solution techniques. Several techniques such as homotopy perturbation method, power series, and Maclaurin series are employed in this analysis. These techniques are rare to analyze the nanomaterial. These techniques have been employed effectively in this analysis and ob-

tained results are compared with the results of other researchers in the existing literature.

1.3. Structure of the dissertation

The dissertation is organized as follows. Section 1 contains the literature review for the nanobeam with different physical and geometrical properties. In section 2, some essential theories, different physical and geometrical properties, and some numerical techniques are presented briefly. In section 3, the effects of rotatory inertia on the natural frequency of nanobeams with steps and cracks are presented. The exact solution technique and homotopy perturbation method are applied for this analysis. In section 4, the dynamic behavior of cracked tapered double nanobeams is analyzed using the Maclaurin series solution technique. In section 5, the transverse vibration of tapered nanobeams with elastic supports is demonstrated. The homotopy perturbation method is used to solve this problem. In section 6, the analysis of the natural frequency of axially graded multi-cracked nanobeams in a thermal environment is presented. The power series solution technique is applied for this analysis. Finally, concluding remarks are presented at the end of the dissertation.

2. GOVERNING EQUATIONS AND ASSUMPTIONS

2.1. Nonlocal theory

Accurate analysis is essential for nanomaterials to utilize their exceptional properties effectively. However, the conventional continuum theory is unable to compute the mechanical behavior of nano-scale materials because it cannot incorporate the size effects of nanomaterials. To overcome the limitations of the existing continuum theory, Eringen [18] proposed the nonlocal theory of elasticity based on assumptions that the stress at a point depends on the strain of all points of the continuum rather than on that point only. According to the assumption, the stress-strain relationship (see Atanasov and Stojanovic [7], Cardoso [10], Malik and Das [54], Martin and Salehian [57]) for a three-dimensional isotropic elastic solid can be expressed as

$$\sigma_{ij}(x) = \int_V f_c(|x - x'|, \tau) t_{ij}(x') dV(x'), \quad (2.1)$$

where

$$t_{ij}(x') = \psi \epsilon_{rr}(x') \delta_{ij} + 2\phi \epsilon_{ij}(x'). \quad (2.2)$$

Here ψ and ϕ represent Lamé's constants. Here $t_{ij}(x')$ and $\epsilon_{ij}(x')$ are the classical stress tensor and linear strain tensor at any point x' in the body, respectively. According to the Hooke's law, the stress-strain relationship can be expressed as

$$t_{ij} = C_{ijkl} \epsilon_{kl}, \quad (2.3)$$

where C_{ijkl} denotes the elastic constants. In equation (2.1), $f_c(|x - x'|, \tau)$ represents the nonlocal modulus to indicate the nonlocal effects into the constitutive equation for the reference point x and source point x' , respectively. Here, $|x' - x|$ represents the Euclidean distance and τ can be presented as

$$\tau = \frac{e_0 a_0}{l}. \quad (2.4)$$

Here τ is a constant that indicates the ratio of internal and external characteristic lengths of the nanomaterial, where e_0 is the material constant. According to the theory of nonlocal elasticity, the integral constitutive relations can be transformed into the equivalent differential equation by substituting the kernel $f_c(|x' - x|, \tau)$ as below

$$(1 - (e_0 a_0)^2 \Delta^2) \sigma_{kl} = t_{kl}. \quad (2.5)$$

In (2.5), Δ^2 denotes the Laplacian operator, σ_{kl} , t_{kl} are the nonlocal and classical stress, respectively. The nonlocal theory depends on the internal characteristic length a_0 . When a_0 becomes zero, the nonlocal constitutive relationship transforms into the classical theory of elasticity. In the one dimensional case, the nonlocal stress-strain relationship can be presented as

$$\sigma(x) - (e_0 a_0)^2 \frac{\partial^2 \sigma(x)}{\partial x^2} = E \epsilon(x), \quad (2.6)$$

where $\sigma(x)$ and $\varepsilon(x)$ represent the stress and strain along the x-axis, E is the modulus of the elasticity. Therefore, the nonlocal elasticity theory can be presented in terms of bending moment M , as follows

$$M - (e_0 a_0)^2 \frac{\partial^2 M}{\partial x^2} = EI \left(-\frac{\partial^2 W}{\partial x^2} \right), \quad (2.7)$$

where W is the deflection of the beam, I indicates the area moment of inertia. According to the nonlocal theory, the equation (2.7) represents the relationship between the bending moment and the deflection.

2.2. Euler-Bernoulli theory

Euler-Bernoulli beam theory is widely used in research and engineering. This theory considers axial and flexural deformation, while the shear deformation is neglected. According to the Euler-Bernoulli's theory, the displacement fields can be expressed as (Malik et al. [53], Yin et al. [80])

$$\begin{aligned} U_1(x, z, t) &= U(x, t) - z \frac{\partial W(x, t)}{\partial x}, \\ U_2(x, z, t) &= 0, \\ U_3(x, z, t) &= W(x, t), \end{aligned} \quad (2.8)$$

where U_1 , U_2 , U_3 express the deflection in x , y and z axis respectively, t indicates the time, $W(x, t)$ and $U(x, t)$ define the transverse and axial deflection, respectively. One can write the axial strain as

$$\varepsilon_{xx} = \frac{\partial U}{\partial x} - z \frac{\partial^2 W}{\partial x^2}. \quad (2.9)$$

Considering the axial load N and moment M , the Euler-Bernoulli equation for vibration can be presented as

$$\frac{\partial^2 M}{\partial x^2} + \frac{\partial}{\partial x} \left(N \frac{\partial W}{\partial x} \right) - m_0 \frac{\partial^2 W}{\partial t^2} = 0, \quad (2.10)$$

where $m_0 = \rho A$ is the mass per unit length.

2.2.1. Transform functions

Basically, the equation of motion is a partial differential equation. Therefore, this partial differential equation can be transformed into an ordinary differential equation using the following function

$$W(x, t) = \bar{W}(x) e^{i\omega_c t}, \quad (2.11)$$

here ω_c is the dimensional frequency.

2.2.2. Non-dimensional parameters

For dimensionless analysis, non-dimensional parameters are essential. Some of the dimensionless parameters are included as follows

$$\xi = \frac{x}{L}, \quad w = \frac{\bar{W}}{L}, \quad \mu = \frac{e_0 a_0}{L}, \quad n = \frac{NL^4}{EI}, \quad \omega^2 = \bar{\omega}^4 = \omega_c^2 L^4 \frac{A\rho}{EI}, \quad (2.12)$$

where ρ is the density of the material, L represents the length of the beam, A stands for the area of the section, I is the moment of inertia of this section, N denotes the axial load and ω_c is the dimensional frequency of natural vibration.

2.2.3. Boundary conditions

To solve the Euler-Bernoulli equation, the required boundary conditions in terms of deflection (W) and moment (M) can be written as follows

for simply supported ends (SS)

$$W(0) = 0, \quad M(0) = 0, \quad W(L) = 0, \quad M(L) = 0, \quad (2.13)$$

for clamped ends (CC)

$$W(0) = 0, \quad W'(0) = 0, \quad W(L) = 0, \quad W'(L) = 0, \quad (2.14)$$

in the case of clamped at the left and simply supported at the right ends (CS)

$$W(0) = 0, \quad W'(0) = 0, \quad W(L) = 0, \quad M(L) = 0, \quad (2.15)$$

for cantilever beams (CF)

$$W(0) = 0, \quad W'(0) = 0, \quad M(L) = 0, \quad M'(L) = \mu^2 \omega^2 W'(L). \quad (2.16)$$

2.3. Modeling of cracks

The basic theory for elastic cracks was established in 1921 by Griffith. According to his theory, crack propagation will occur if the energy is sufficient to provide all the energy that is required for crack growth. The mechanical properties of open and stable cracks can be analyzed using the classical local stiffness model. According to the model, the beam is separated into two sub-beams for each crack at the crack position. The stiffness of the rotational spring depends on strain energy release rate as well as the crack depth. The strain energy S of the cracked beam in terms of axial force and bending moment can be expressed as

$$S = \frac{1}{2} \int_0^L \left(N \frac{\partial U}{\partial x} + M \frac{\partial^2 W}{\partial x^2} \right) dx + \Delta s_c. \quad (2.17)$$

Due to the presence of a crack, the beam has additional strain energy to the rotational spring. Strain energy is caused by the bending moment and axial stress. The change of strain energy (Roostai and Haghpanahi [66]) can be presented as

$$\Delta s_c = \frac{1}{2}M\Delta\theta + \frac{1}{2}N\Delta u, \quad (2.18)$$

where $\Delta\theta$ indicates the rotational angle because of the spring and Δu represents the axial displacement at the location of the crack. Therefore, these can be expressed as

$$\begin{aligned} \Delta\theta &= K_{MM} \frac{\partial^2 W}{\partial x^2} + K_{MN} \frac{\partial U}{\partial x}, \\ \Delta u &= K_{NN} \frac{\partial U}{\partial x} + K_{NM} \frac{\partial^2 W}{\partial x^2}, \end{aligned} \quad (2.19)$$

where K_{MM} , K_{MN} , K_{NN} , K_{NM} are the flexibility constants. When the beam is subjected to transverse vibration, longitudinal displacement Δu and the flexibility constants K_{MN} , K_{NN} , K_{NM} are small which can be neglected. Therefore, the discontinuity at the location of the crack can be expressed as

$$\Delta\theta = \frac{K_{MM}}{L} \frac{\partial^2 W(x)}{\partial x^2} \Big|_{x=a}, \quad (2.20)$$

where $\frac{K_{MM}}{L}$ can be replaced by K as

$$\Delta\theta = K \frac{\partial^2 W(x)}{\partial x^2} \Big|_{x=a}, \quad (2.21)$$

here K is the crack severity. The crack severity depends on the crack depth and rotational spring stiffness. It can be expressed as follows

$$K = \frac{EI}{L} \frac{1}{\kappa_s}. \quad (2.22)$$

Here κ_s is the spring stiffness. It can be written as

$$\kappa_s = \frac{EI}{h} \frac{1}{C(s)}, \quad (2.23)$$

here $s = \frac{c}{h}$ is the ratio of crack depth (c) and beam height (h). In (2.23), $C(s)$ is the local compliance that can be computed from the strain energy density function as

$$\begin{aligned} C(s) &= 5.346(1.86s^2 - 3.95s^3 + 16.375s^4 - 37.226s^5 + 76.81s^6 \\ &\quad - 126.9s^7 + 172s^8 - 143.97s^9 + 66.56s^{10}). \end{aligned} \quad (2.24)$$

The crack severity can be written according to equations (2.22) and (2.23) as

$$K = \frac{h}{L} C(s). \quad (2.25)$$

Crack severity doesn't depend on the modulus of elasticity and density of the beam. Therefore, crack severity K is constant over the length of the axially graded beam. Another way, it can be presented (see Hsu [37]) as

$$K = \frac{6(1 - \nu^2)h}{EI} f(s), \quad (2.26)$$

where

$$f(s) = \int_0^s \pi s F^2(s) ds, \quad (2.27)$$

and ν is the Poisson ratio. The factor F can be defined by trigonometric or algebraic functions (see Tada [76]) as

$$F(s) = \sqrt{\frac{2}{\pi s} \tan\left(\frac{\pi s}{2}\right)} \frac{0.923 + 0.199[1 - \sin\left(\frac{\pi s}{2}\right)]^4}{\cos\left(\frac{\pi s}{2}\right)}, \quad (2.28)$$

and

$$F(s) = 1.93 - 3.07s + 14.53s^2 - 25.11s^3 + 25.8s^4, \quad (2.29)$$

where

$$s = \frac{c}{h}. \quad (2.30)$$

Intermediate conditions for steps and cracks are as follows

$$\begin{aligned} w_i(x_i) &= w_{i+1}(x_i), \\ w_i''(x_i) &= w_{i+1}''(x_i), \\ w_{i+1}'(x_i) - w_i'(x_i) &= K w_i''(x_i), \\ w_i'''(x_i) + \psi_i w_i'(x_i) &= w_{i+1}'''(x_i) + \psi_{i+1} w_{i+1}'(x_i), \end{aligned} \quad (2.31)$$

where $\psi = \omega^2 \mu^2$.

2.4. Tapered beams

Tapered beams offer us an optimized design by saving material consumption. In this dissertation, the beam is tapered along the x-axis where the width of the beam is varying exponentially from one end to another end. The variable quantities such as width, area, moment of inertia can be expressed as (see Hossain and Lellep [32], Kalkowski et al. [39], Mahmoud [52])

$$\begin{aligned} b(x) &= b_0 e^{\alpha \frac{x}{L}}, \\ A(x) &= A_0 e^{\alpha \frac{x}{L}}, \\ I(x) &= I_0 e^{\alpha \frac{x}{L}}, \end{aligned} \quad (2.32)$$

where α is the taper ratio of the beam, it indicates the variation of width along the x-axis and b_0 , A_0 , I_0 are the width, area, moment of inertia at $x = 0$, respectively. The area and area moment of inertia are also varying with the width of the beam.

2.5. Axially graded beams

An axially graded beam is a non-homogeneous beam. It is also a type of composite (Li and Liu [46]) that can avoid the point of stress concentration. It distributes stress gradually from one end to another end. Along the cross-section, material properties are constant. It is assumed that the modulus of elasticity E and density ρ are varying along the length exponentially (Mamaghani et al. [55]). These can be expressed as

$$E(x) = E_0 e^{\lambda \frac{x}{L}}, \quad \rho(x) = \rho_0 e^{\lambda \frac{x}{L}}, \quad (2.33)$$

where E_0 and ρ_0 are the modulus of elasticity and density at the point $x = 0$, λ is the coefficient of non-homogeneity.

2.6. Thermal loading

The thermal load is the result of temperature change which is induced by the contraction and expansion of the material. This load acts as an axial load on the beam element. Thermal load can be expressed as

$$N = -EA\alpha_t\theta, \quad (2.34)$$

where α_t is the coefficient of thermal expansion, θ is the temperature change and A stands for the area of the cross-section. According to the theory of Zarzycki 1982 and Kittel 1983, the coefficient of thermal expansion can be presented as

$$\alpha_t = \frac{\gamma_G \rho c_v}{3E}, \quad (2.35)$$

where γ_G is the Grüneisen constant and c_v stands for the specific heat of the material. Here, α_t depends on density and modulus of elasticity. Here, in the axially graded beam, both of these parameters are varying equally along the x-axis. Therefore, α_t is constant over the length.

2.7. Homotopy perturbation method

The homotopy perturbation method is a very useful technique to analyze linear and nonlinear problems. He (see He [25-27]) proposed this method in 1998. The homotopy perturbation method is the combination of the perturbation method and homotopy method. In this method, small parameters are not essential. An early approximation can be selected freely with possible unknown constants. Finally, the obtained approximations are valid for small parameters as well as very large parameters. Applying the homotopy perturbation method for solving nonlinear differential equations, one can consider the following nonlinear differential equation

$$D(w) - f(r) = 0 \quad (r \in \Omega), \quad (2.36)$$

and boundary conditions can be described as follows

$$B(w, \frac{\partial w}{\partial \xi}) = 0 \quad (r \in \Gamma), \quad (2.37)$$

where D is a general differential operator, B is a boundary operator, $f(r)$ is a known analytical function, Γ is the boundary of the solution domain (Ω), and $\frac{\partial w}{\partial \xi}$ denotes differential along the outward normal to Γ . Basically, the operator D can be divided into two parts, a linear part L and a nonlinear one N . Therefore, the equation (2.36) can be written as follows

$$L(w) + N(w) - f(r) = 0. \quad (2.38)$$

In cases where the nonlinear equation (2.38) includes no small parameter, one can construct the following homotopy equation

$$H(w, P) = L(w) + P[N(w) - f(r)] = 0. \quad (2.39)$$

In the equation (2.39), $P \in [0, 1]$ is the embedded parameter and w_0 is the first approximation that satisfies the boundary condition. One may assume that solution of the equation (2.39) can be written as a power series of P , as given below

$$w = w_0 + Pw_1 + P^2w_2 + \dots \quad (2.40)$$

Considering $P = 1$, equation (2.40) can be written as

$$w = w_0 + w_1 + w_2 + \dots \quad (2.41)$$

Using the given fourth order differential governing equation (nano elements), one can calculate the first term of the series as follows

$$w_0 = B_0 + B_1\xi + \frac{1}{2}B_2\xi^2 + \frac{1}{6}B_3\xi^3, \quad (2.42)$$

here, B_0, \dots, B_3 are constant coefficients of the series. Similarly, one can calculate the other terms (w_1, w_2, \dots, w_n) of the series. Therefore, using the boundary conditions, the series solution can be transformed into matrix form as follows

$$[M] \begin{bmatrix} B_0 \\ B_1 \\ B_2 \\ B_3 \end{bmatrix} = 0. \quad (2.43)$$

Solving this linear algebraic equation, one can get the non-trivial solution for the governing equation.

2.8. Maclaurin series

Maclaurin series is a special case of a Taylor series around $x_0 = 0$. This series represents the approximate solution of a function $f(x)$. Therefore, this series can be expressed as

$$f(x) = \sum_{n=0}^{\infty} \frac{f^{(n)}(0)}{n!} x^n = f(0) + f'(0)x + \frac{f''(0)}{2!}x^2 + \dots + \frac{f^{(k)}(0)}{k!}x^k + \dots, \quad (2.44)$$

it can be considered as follows

$$\begin{aligned} f(0) &= C_0, \\ f'(0) &= C_1, \\ f''(0) &= C_2, \\ f'''(0) &= C_3. \end{aligned} \quad (2.45)$$

Similarly, using the given governing equation $f^4(0)$, $f^5(0)$, ..., $f^n(0)$ can be presented by the constants (C_0, C_1, \dots, C_3) , then the solution becomes

$$f(x) = C_0 + C_1x + C_2\frac{x^2}{2} + C_3\frac{x^3}{6} + \dots \quad (2.46)$$

Using the boundary conditions, solution (2.46) can be transformed into matrix form as follows

$$[M] \begin{bmatrix} C_0 \\ C_1 \\ C_2 \\ C_3 \end{bmatrix} = 0. \quad (2.47)$$

Solving the equation (2.47), one can find the non-trivial solution.

2.9. Power series

The power series solution (Soltani et al. [71]) is a semi-analytical technique. In this technique, a power series is considered as a solution of the function. Therefore, the differential equation transforms into an algebraic equation. Linear and nonlinear differential equations can be solved using the power series solution technique. Let us consider a series for the deflection of the beam as

$$w = \sum_{k=0}^{\infty} A_k \xi^k, \quad (2.48)$$

here, A_k represent the constant coefficients of the series. Similarly, derivatives of deflection can be written as

$$\begin{aligned}\frac{dw}{d\xi} &= \sum_{k=1}^{\infty} k A_k \xi^{k-1}, \\ \frac{d^2w}{d\xi^2} &= \sum_{k=2}^{\infty} k(k-1) A_k \xi^{k-2}, \\ \frac{d^3w}{d\xi^3} &= \sum_{k=3}^{\infty} k(k-1)(k-2) A_k \xi^{k-3}, \\ \frac{d^4w}{d\xi^4} &= \sum_{k=4}^{\infty} k(k-1)(k-2)(k-3) A_k \xi^{k-4}.\end{aligned}\tag{2.49}$$

Using the governing equation, it is needed to make a recursive relationship between coefficients as follows

$$A_{k+4} = g_1 A_{k+3} + g_2 A_{k+2} + g_3 A_{k+1} + g_4 A_k,\tag{2.50}$$

here, g_1, \dots, g_4 are the constants come from the given governing equation. Applying this relationship, one can calculate $A_4, A_5, A_6, \dots, A_n$ in terms of A_0, A_1, \dots, A_3 . Therefore, one can write the series solution as follows

$$w = A_0 + A_1 \xi + A_2 \xi^2 + A_3 \xi^3 + \dots \quad .\tag{2.51}$$

Using the boundary conditions, this series solution can be transformed into matrix form as below

$$[M] \begin{bmatrix} A_0 \\ A_1 \\ A_2 \\ A_3 \end{bmatrix} = 0.\tag{2.52}$$

Solving this equation, one can get the non-trivial solution.

3. THE EFFECT OF ROTATORY INERTIA ON THE NATURAL FREQUENCY OF CRACKED AND STEPPED NANOBEAMS

In this section, the effect of rotatory inertia is considered in the analysis of the dynamic behavior of the cracked and stepped nanobeams. Usually, in the Euler-Bernoulli beam theory, the effect of rotatory inertia is ignored. Here, it is taken into account. In this section, the Euler-Bernoulli beam theory combined with rotatory inertia is solved using two techniques such as the exact solution method in Hossain and Lellep [28, 29] and the homotopy perturbation method in Hossain and Lellep [33].

3.1. Formulation of the problem

A schematic shape of a stepped and cracked nanobeams is illustrated in Figure 1. It is assumed that the origin of the coordinate system is placed at the left corner point of the nanobeam. The axis of the beam coincides with the x-axis and the height of the beam along the z-axis. The nanobeam with length L and rectangular cross-section of width b is considered. The density of the material ρ is constant throughout the beam and the axial load N is also applied uniformly over the nanobeam. The crack and step are placed at the same location at $x = a$ of

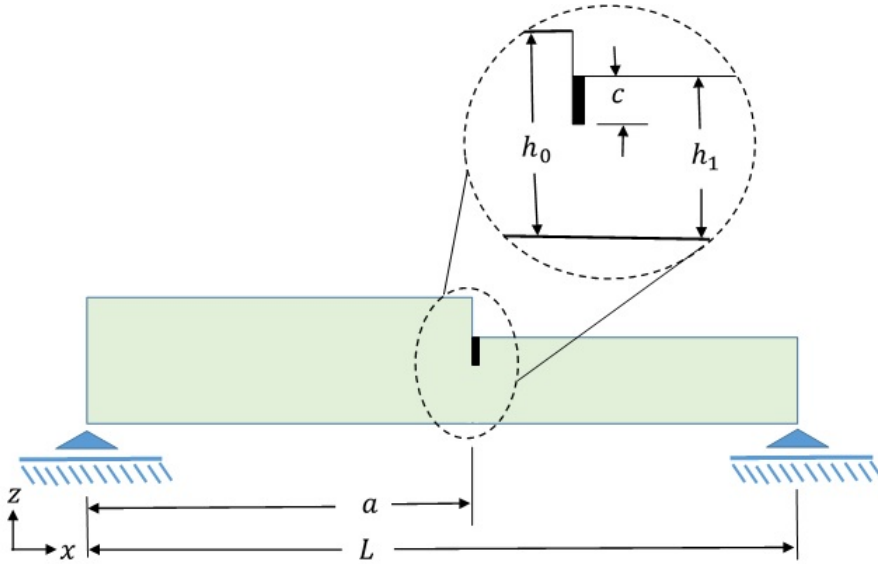


Figure 1. Geometry of the cracked and stepped nanobeam

the beam. So, there is a discontinuity at that location with different heights

$$h = \begin{cases} h_0, & x \in (0, a) \\ h_1, & x \in (a, L) \end{cases}. \quad (3.1)$$

The crack is treated as a stable crack whose depth is c (s is the ratio of crack depth and beam height).

3.2. Mathematical model

The equilibrium equation for the Euler–Bernoulli beam theory with rotatory inertia can be written as (see Reddy [65])

$$\frac{\partial^2 M}{\partial x^2} + N \frac{\partial^2 W}{\partial x^2} = I_0 \frac{\partial^2 W}{\partial t^2} - I_r \frac{\partial^4 W}{\partial x^2 \partial t^2}, \quad (3.2)$$

in which I_0 presents the second moment of area and E is the modulus of elasticity. N denotes the axial force that is considered constant throughout the beam. I_r represents the rotatory inertia as $I_r = \rho \int_{-h/2}^{h/2} z^2 dA$, ρ is the density of the material and t is the time. Applying the nonlocal theory (2.7) and Euler–Bernoulli beam theory (3.2), the governing equation for nanobeams can be written as

$$\begin{aligned} EI \frac{\partial^4 W}{\partial x^4} - (e_0 a)^2 \left(-N \frac{\partial^4 W}{\partial x^4} + I_0 \frac{\partial^4 W}{\partial x^2 \partial t^2} - I_r \frac{\partial^6 W}{\partial x^4 \partial t^2} \right) \\ - N \frac{\partial^2 W}{\partial x^2} + I_0 \frac{\partial^2 W}{\partial t^2} - I_r \frac{\partial^4 W}{\partial x^2 \partial t^2} = 0. \end{aligned} \quad (3.3)$$

Using the transformation (2.11) and non-dimensional parameters (2.12), the equation (3.3) can be simplified as follows

$$(1 + \mu^2 n - \mu^2 r_0 \omega_0^2) \frac{d^4 w}{d\xi^4} + (\mu^2 \omega_0^2 - n + r_0 \omega_0^2) \frac{d^2 w}{d\xi^2} - \omega_0^2 w = 0, \quad (3.4)$$

where

$$I_0 = \rho, \quad r_0 = \frac{1}{12} \left(\frac{h_0}{L} \right)^2, \quad \gamma = \frac{h_1}{h_0}, \quad \omega_1 = \gamma \omega_0.$$

Here, r_0 indicate the effect of rotatory inertia, γ is the step heights ratio, h_0 , h_1 are the height of the two segments, respectively. ω_0 , ω_1 are the dimensionless frequency for two different segments of the beam. Considering the two segments of the beam, the equation (3.4) can be written as follows

$$\frac{d^4 w}{d\xi^4} + \frac{\beta_0}{\alpha_0} \frac{d^2 w}{d\xi^2} - \frac{\omega_0^2}{\alpha_0} w = 0, \quad \text{for } \xi \in (0, a), \quad (3.5)$$

and

$$\frac{d^4 w}{d\xi^4} + \frac{\beta_1}{\alpha_1} \frac{d^2 w}{d\xi^2} - \frac{\omega_1^2}{\alpha_1} w = 0, \quad \text{for } \xi \in (a, 1), \quad (3.6)$$

where

$$\alpha_0 = 1 + \mu^2 n - \mu^2 r_0 \omega_0^2, \quad \beta_0 = \mu^2 \omega_0^2 - n + r_0 \omega_0^2,$$

and

$$\alpha_1 = 1 + \mu^2 n - \mu^2 r_1 \omega_1^2, \quad \beta_1 = \mu^2 \omega_1^2 - n + r_1 \omega_1^2.$$

3.3. Solution

Using the basic principle of the homotopy perturbation method (HPM), we can write the governing equation for $x \in (0, a)$ as

$$(1 - P) \left(\frac{d^4 w}{d\xi^4} - \frac{d^4 w_0}{d\xi^4} \right) + P \left(\frac{d^4 w}{d\xi^4} + \frac{\beta_0}{\alpha_0} \frac{d^2 w}{d\xi^2} - \frac{\omega_0^2}{\alpha_0} w \right) = 0, \quad (3.7)$$

for $x \in (a, L)$ as

$$(1 - P) \left(\frac{d^4 w}{d\xi^4} - \frac{d^4 w_0}{d\xi^4} \right) + P \left(\frac{d^4 w}{d\xi^4} + \frac{\beta_1}{\alpha_1} \frac{d^2 w}{d\xi^2} - \frac{\omega_1^2}{\alpha_1} w \right) = 0, \quad (3.8)$$

here, P is the perturbation constant. We consider the initial approximations (w_0) for two different segments as follows

$$w_0 = A_0 + \xi A_1 + \frac{1}{2} \xi^2 A_2 + \frac{1}{6} \xi^3 A_3, \quad x \in (0, a) \quad (3.9)$$

and

$$w_0 = A_4 + \xi A_5 + \frac{1}{2} \xi^2 A_6 + \frac{1}{6} \xi^3 A_7, \quad x \in (a, 1), \quad (3.10)$$

here, A_0, \dots, A_7 are constants. Using the initial approximation, one can calculate the other terms ($w_1, w_2, w_3, \dots, w_n$) of the governing equations. Using the boundary and intermediate conditions, eight algebraic equations are formed with eight unknowns. These equations can be presented in a matrix form as

$$[M(\omega_0)] \begin{bmatrix} A_0 \\ A_2 \\ A_3 \\ \dots \\ A_6 \\ A_7 \end{bmatrix} = 0. \quad (3.11)$$

Here, M is the coefficient matrix. Solving the determinant of the matrix, one can find the nontrivial solution. The real value of ω_0 represents the natural frequency of nanobeams. This problem is solved using the exact solution technique (see Hossain and Lellep [28-31], Motaghiana et al. [58]). The results of both techniques are compared favorably.

Table 1. The frequency of nanobeams for different values of the nonlocal parameter

μ	Mode	SS		CS		CC	
		HPM	Exact	HPM	Exact	HPM	Exact
0	1	9.981445	9.869384	15.51445	15.41723	22.51406	22.37353
0	2	39.80859	39.47705	50.22031	49.96533	61.95312	61.67138
0	3	90.64375	88.82519	107.1218	104.2412	125.6960	120.9042
0.5	1	5.406738	5.300292	7.942382	7.784423	11.21069	10.99121
0.5	2	12.21386	11.97485	14.98398	14.68701	24.80932	24.33398
0.5	3	18.803 71	18.43798	21.68398	21.25610	31.07802	30.48632

Table 2. The frequency of nanobeams for different values of length to height ratio

L/h	Mode	SS		CS		CC	
		HPM	Exact	HPM	Exact	HPM	Exact
100	1	9.607031	9.411523	14.88769	14.58496	21.50297	21.06894
100	2	33.81835	33.26757	42.24589	41.48828	51.34238	50.434 17
100	3	65.35546	63.53564	74.71484	73.15234	84.91113	83.19316
20	1	9.511230	9.321679	14.55371	14.27050	20.55322	20.17104
20	2	30.44335	30.03339	36.18554	35.70507	41.46464	40.974 80
20	3	47.87109	47.13593	51.24609	50.73046	54.23222	53.77070

3.4. Results and discussion

In this section, the dynamic behavior of stepped and cracked nanobeams is solved by the homotopy perturbation method (HPM) and the exact solution method. At first, the accuracy of the analysis is measured by comparing the results of HPM with the results of the exact solution technique. Then, the effect of nonlocal parameter, step and crack location, and crack depth on natural frequency are presented for different boundary conditions such as simply supported (SS), clamped simply (CS), and fully clamped (CC). The results of the calculation are described in Tables 1, 2 and Figures 2–5.

Table 1 presents the natural frequency for different support systems. Different modes of frequency and different values of the nonlocal parameter are also considered. It is clear from this table that the natural frequency decreases rapidly with the increase of the nonlocal parameter. Table 2 illustrates the effect of rotatory inertia on the natural frequency of nanobeams under different support systems. It is clearly described that natural frequency decreases with the decrease of the length to height ratio. Thus the effect of rotatory inertia can be ignored in a slender beam.

Figures 2, and 3 describe the effect of crack depth on the natural frequency of nanobeams for different modes of frequency and different boundary conditions such as fully clamped (CC), and clamped simply (CS). It can be seen from these graphs that natural frequency decreases with the increase of crack depth. In the first mode, when the crack depth is more than 0.4 then rotatory inertia has no

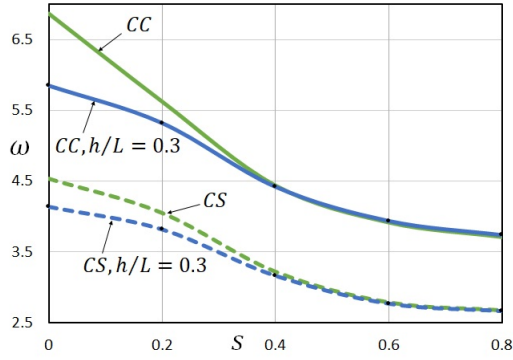


Figure 2. Frequency versus crack depth in first mode

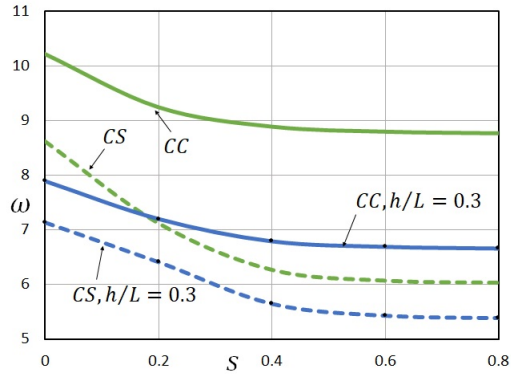


Figure 3. Frequency versus crack depth in second mode

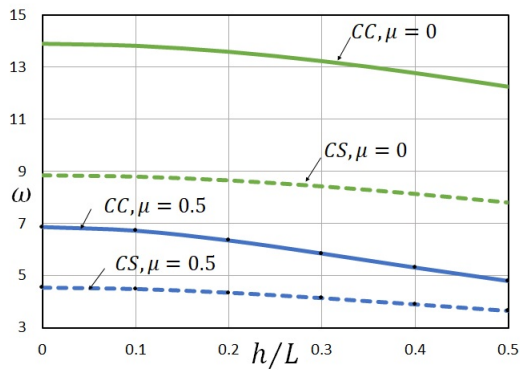


Figure 4. Frequency versus height to length ratio in first mode

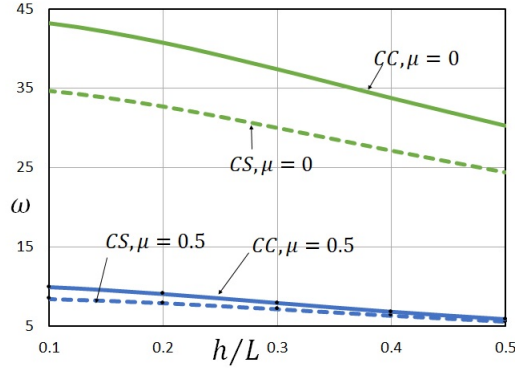


Figure 5. Frequency versus height to length ratio in second mode

effect. On the other hand, rotatory inertia is more significant in the second mode than in the first mode of frequency.

Figures 4, and 5 describe the effect of the height to length ratio on the natural frequency of nanobeams for different modes of frequency and different boundary conditions such as fully clamped (CC), and clamped simply (CS). It is clear from these graphs that natural frequency decreases with the increase of the height to length ratio. In the second mode of frequency, the natural frequency decreases more rapidly than in the first mode of frequency.

4. FREE VIBRATION ANALYSIS OF THE TAPERED CRACKED DOUBLE NANOBEAMS USING MACLAURIN SERIES

In this section, the Maclaurin series technique is applied to analyze the vibration of the cracked tapered double nanobeams according to the paper by Hossain and Lellep [35].

4.1. Physical description

A schematic shape of a double nanobeam (see Li et al. [50]) consists of two parallel nanobeams connected to each other by distributed vertical Winkler-type springs. The two nanobeams are referred to as the upper beam and the lower beam. Nanobeams are assumed to be slender and satisfy the Euler-Bernoulli beam theory. The origin of the coordinate system (x, y, z) is considered to be at the left endpoint of the beam. Both nanobeams are assumed to be made of the same material. Therefore, physical and geometrical properties are considered to be the same for both beams. A crack is considered as a defect on the upper beam.

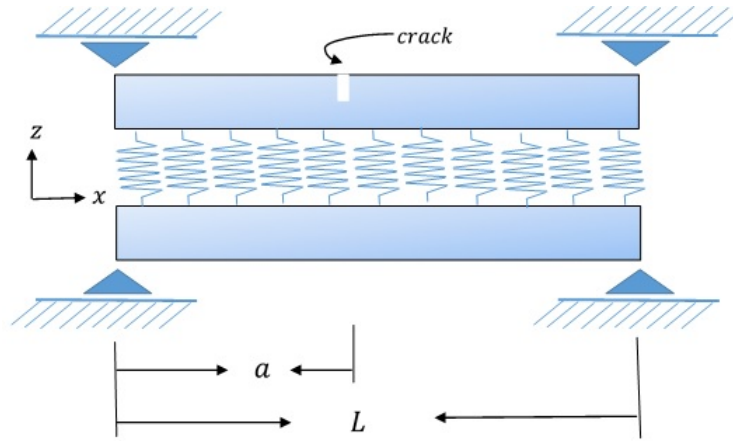


Figure 6. A cracked tapered double nanobeam

4.1.1. Governing equations

A double beam consists of two beams with identical material and geometric properties. Combining the classical beam theory (2.10) and the nonlocal theory of

elasticity (2.7), coupled equations for the double nanobeam can be written as

$$\begin{aligned}
& \frac{\partial^2}{\partial x^2} \left(EI(x) \frac{\partial^2 W_1}{\partial x^2} \right) + \rho A(x) \frac{\partial^2 W_1}{\partial t^2} - (e_0 a_0)^2 \frac{\partial^2}{\partial x^2} \left(\rho A(x) \frac{\partial^2 W_1}{\partial t^2} \right) \\
& + K_s \frac{b(x)}{b_0} (W_1 - W_2) - (e_0 a_0)^2 K_s \frac{\partial^2}{\partial x^2} \frac{b(x)}{b_0} (W_1 - W_2) = 0, \\
& \frac{\partial^2}{\partial x^2} \left(EI(x) \frac{\partial^2 W_2}{\partial x^2} \right) + \rho A(x) \frac{\partial^2 W_2}{\partial t^2} - (e_0 a_0)^2 \frac{\partial^2}{\partial x^2} \left(\rho A(x) \frac{\partial^2 W_2}{\partial t^2} \right) \\
& + K_s \frac{b(x)}{b_0} (W_2 - W_1) - (e_0 a_0)^2 K_s \frac{\partial^2}{\partial x^2} \frac{b(x)}{b_0} (W_2 - W_1) = 0,
\end{aligned} \tag{4.1}$$

where W_1 and W_2 are the deflection of upper and lower beams in the double nanobeam, respectively. K_s is the spring constant. Both beams in the double beam are tapered along the x -axis. Applying geometric properties of the tapered beam (2.32), the equation (4.1) can be expressed as

$$\begin{aligned}
& \frac{\partial^2}{\partial x^2} \left(EI_0 e^{\frac{\alpha x}{L}} \frac{\partial^2 W_1}{\partial x^2} \right) + \rho A_0 e^{\frac{\alpha x}{L}} \frac{\partial^2 W_1}{\partial t^2} - (e_0 a_0)^2 \frac{\partial^2}{\partial x^2} \left(\rho A_0 e^{\frac{\alpha x}{L}} \frac{\partial^2 W_1}{\partial t^2} \right) \\
& + K_s \frac{b_0}{b_0} e^{\frac{\alpha x}{L}} (W_1 - W_2) - (e_0 a_0)^2 K_s \frac{\partial^2}{\partial x^2} \frac{b_0}{b_0} e^{\frac{\alpha x}{L}} (W_1 - W_2) = 0, \\
& \frac{\partial^2}{\partial x^2} \left(EI_0 e^{\frac{\alpha x}{L}} \frac{\partial^2 W_2}{\partial x^2} \right) + \rho A_0 e^{\frac{\alpha x}{L}} \frac{\partial^2 W_2}{\partial t^2} - (e_0 a_0)^2 \frac{\partial^2}{\partial x^2} \left(\rho A_0 e^{\frac{\alpha x}{L}} \frac{\partial^2 W_2}{\partial t^2} \right) \\
& + K_s \frac{b_0}{b_0} e^{\frac{\alpha x}{L}} (W_2 - W_1) - (e_0 a_0)^2 K_s \frac{\partial^2}{\partial x^2} \frac{b_0}{b_0} e^{\frac{\alpha x}{L}} (W_2 - W_1) = 0,
\end{aligned} \tag{4.2}$$

where α is the tapered ratio. These partial differential equations can be transformed into ordinary differential equations using the transformation (2.11) and dimensionless parameters (2.12). Therefore, the equation (4.2) can be presented as

$$\begin{aligned}
& \frac{d^4 w_1}{d\xi^4} + 2\alpha \frac{d^3 w_1}{d\xi^3} + (\mu^2 \omega^2 - k_s \mu^2 + \alpha^2) \frac{d^2 w_1}{d\xi^2} + (2\alpha \mu^2 \omega^2 - 2\alpha k_s \mu^2) \frac{dw_1}{dx} \\
& + (\alpha^2 \mu^2 \omega^2 - \alpha^2 k_s \mu^2 - \omega^2 + k_s) w_1 + \mu^2 k_s \frac{d^2 w_2}{dx^2} \\
& + 2\mu^2 k_s \alpha \frac{dw_2}{dx} + (\alpha^2 k_s \mu^2 - k_s) w_2 = 0, \\
& \frac{d^4 w_2}{d\xi^4} + 2\alpha \frac{d^3 w_2}{d\xi^3} + (\mu^2 \omega^2 - k_s \mu^2 + \alpha^2) \frac{d^2 w_2}{d\xi^2} + (2\alpha \mu^2 \omega^2 - 2\alpha k_s \mu^2) \frac{dw_2}{dx} \\
& + (\alpha^2 \mu^2 \omega^2 - \alpha^2 k_s \mu^2 - \omega^2 + k_s) w_2 + \mu^2 k_s \frac{d^2 w_1}{dx^2} \\
& + 2\mu^2 k_s \alpha \frac{dw_1}{dx} + (\alpha^2 k_s \mu^2 - k_s) w_1 = 0,
\end{aligned} \tag{4.3}$$

here, w_1, w_2 are the dimensionless deflection of upper and lower beams, $k_s = \frac{K_s L^4}{EI}$ is the dimensionless spring constant. These equations (4.3) are the governing

equations of the double nanobeam in simplified form. These equations can be solved numerically.

4.2. Solution technique

Deformations of a double nanobeam are presented by coupled differential equations (4.3). These governing equations can be presented as follows

$$\begin{aligned}
\frac{d^4 w_1}{d\xi^4} &= -2\alpha \frac{d^3 w_1}{d\xi^3} - (\mu^2 \omega^2 - k_s \mu^2 + \alpha^2) \frac{d^2 w_1}{d\xi^2} \\
&\quad - (2\alpha \mu^2 \omega^2 - 2\alpha k_s \mu^2) \frac{dw_1}{d\xi} - (\alpha^2 \mu^2 \omega^2 - \alpha^2 k_s \mu^2 \\
&\quad - \omega^2 + k_s) w_1 - \mu^2 k_s \frac{d^2 w_2}{d\xi^2} - 2\mu^2 k_s \alpha \frac{dw_2}{d\xi} - (\alpha^2 k_s \mu^2 - k_s) w_2, \\
\frac{d^4 w_2}{d\xi^4} &= -2\alpha \frac{d^3 w_2}{d\xi^3} - (\mu^2 \omega^2 - k_s \mu^2 + \alpha^2) \frac{d^2 w_2}{d\xi^2} \\
&\quad - (2\alpha \mu^2 \omega^2 - 2\alpha k_s \mu^2) \frac{dw_2}{d\xi} - (\alpha^2 \mu^2 \omega^2 - \alpha^2 k_s \mu^2 \\
&\quad - \omega^2 + k_s) w_2 - \mu^2 k_s \frac{d^2 w_1}{d\xi^2} - 2\mu^2 k_s \alpha \frac{dw_1}{d\xi} - (\alpha^2 k_s \mu^2 - k_s) w_1.
\end{aligned} \tag{4.4}$$

Differentiating the equations (4.4), one can express as

$$\begin{aligned}
\frac{d^5 w_1}{d\xi^5} &= -2\alpha \frac{d^4 w_1}{d\xi^4} - (\mu^2 \omega^2 - k_s \mu^2 + \alpha^2) \frac{d^3 w_1}{d\xi^3} \\
&\quad - (2\alpha \mu^2 \omega^2 - 2\alpha k_s \mu^2) \frac{d^2 w_1}{d\xi^2} - (\alpha^2 \mu^2 \omega^2 - \alpha^2 k_s \mu^2 - \omega^2 + k_s) \frac{dw_1}{d\xi} \\
&\quad - \mu^2 k_s \frac{d^3 w_2}{d\xi^3} - 2\mu^2 k_s \alpha \frac{d^2 w_2}{d\xi^2} - (\alpha^2 k_s \mu^2 - k_s) \frac{dw_2}{d\xi}, \\
\frac{d^5 w_2}{d\xi^5} &= -2\alpha \frac{d^4 w_2}{d\xi^4} - (\mu^2 \omega^2 - k_s \mu^2 + \alpha^2) \frac{d^3 w_2}{d\xi^3} \\
&\quad - (2\alpha \mu^2 \omega^2 - 2\alpha k_s \mu^2) \frac{d^2 w_2}{d\xi^2} - (\alpha^2 \mu^2 \omega^2 - \alpha^2 k_s \mu^2 - \omega^2 + k_s) \frac{dw_2}{d\xi} \\
&\quad - \mu^2 k_s \frac{d^3 w_1}{d\xi^3} - 2\mu^2 k_s \alpha \frac{d^2 w_1}{d\xi^2} - (\alpha^2 k_s \mu^2 - k_s) \frac{dw_1}{d\xi}.
\end{aligned} \tag{4.5}$$

Substituting $\frac{d^4 w_1}{d\xi^4}$ and $\frac{d^4 w_2}{d\xi^4}$ in the equations (4.5), one can express as

$$\begin{aligned}
\frac{d^5 w_1}{d\xi^5} &= (-\mu^2 \omega^2 + k_s \mu^2 + 3\alpha^2) \frac{d^3 w_1}{d\xi^3} + 2\alpha^3 \frac{d^2 w_1}{d\xi^2} \\
&+ (3\alpha^2 \mu^2 \omega^2 - 3\alpha^2 k_s \mu^2 + \omega^2 - k_s) \frac{dw_1}{d\xi} - 2\alpha(-\alpha^2 \mu^2 \omega^2 + \alpha^2 k_s \mu^2 \\
&+ \omega^2 - k_s) w_1 - k_s \mu^2 \frac{d^3 w_2}{d\xi^3} + (3\alpha^2 k_s \mu^2 + k_s) \frac{dw_2}{d\xi} + 2\alpha k_s (\alpha^2 \mu^2 - 1) w_2, \\
\frac{d^5 w_2}{d\xi^5} &= (-\mu^2 \omega^2 + k_s \mu^2 + 3\alpha^2) \frac{d^3 w_2}{d\xi^3} + 2\alpha^3 \frac{d^2 w_2}{d\xi^2} \\
&+ (3\alpha^2 \mu^2 \omega^2 - 3\alpha^2 k_s \mu^2 + \omega^2 - k_s) \frac{dw_2}{d\xi} - 2\alpha(-\alpha^2 \mu^2 \omega^2 + \alpha^2 k_s \mu^2 \\
&+ \omega^2 - k_s) w_2 - k_s \mu^2 \frac{d^3 w_1}{d\xi^3} + (3\alpha^2 k_s \mu^2 + k_s) \frac{dw_1}{d\xi} + 2\alpha k_s (\alpha^2 \mu^2 - 1) w_1.
\end{aligned} \tag{4.6}$$

Similarly, one can calculate $\frac{d^6 w_1}{d\xi^6}$, $\frac{d^6 w_2}{d\xi^6}$, $\frac{d^7 w_1}{d\xi^7}$, $\frac{d^7 w_2}{d\xi^7}$, ..., upto n^{th} order derivative. Let us consider the values of derivatives at zero as follows

$$\begin{aligned}
w_1(0) &= c_0, \quad w_2(0) = d_0, \\
\frac{dw_1}{d\xi}(0) &= c_1, \quad \frac{dw_2}{d\xi}(0) = d_1, \\
\frac{d^2 w_1}{d\xi^2}(0) &= c_2, \quad \frac{d^2 w_2}{d\xi^2}(0) = d_2, \\
\frac{d^3 w_1}{d\xi^3}(0) &= c_3, \quad \frac{d^3 w_2}{d\xi^3}(0) = d_3.
\end{aligned} \tag{4.7}$$

Using the equations (4.7), higher derivatives at zero can be expressed as

$$\begin{aligned}
\frac{d^4 w_1}{d\xi^4}(0) &= (-\alpha^2 \mu^2 \omega^2 + \alpha^2 k_s \mu^2 + \omega^2 - k_s) c_0 \\
&+ (-2\alpha \mu^2 \omega^2 + 2\alpha k_s \mu^2) c_1 + (-\mu^2 \omega^2 + k_s \mu^2 - \alpha^2) c_2 + (-2\alpha) c_3 \\
&+ (-\alpha^2 k_s \mu^2 + k_s) d_0 + (-2\alpha k_s \mu^2) d_1 + (-k_s \mu^2) d_2, \\
\frac{d^4 w_2}{d\xi^4}(0) &= (-\alpha^2 \mu^2 \omega^2 + \alpha^2 k_s \mu^2 + \omega^2 - k_s) d_0 \\
&+ (-2\alpha \mu^2 \omega^2 + 2\alpha k_s \mu^2) d_1 + (-\mu^2 \omega^2 + k_s \mu^2 - \alpha^2) d_2 + (-2\alpha) d_3 \\
&+ (-\alpha^2 k_s \mu^2 + k_s) c_0 + (-2\alpha k_s \mu^2) c_1 + (-k_s \mu^2) c_2, \\
\frac{d^5 w_1}{d\xi^5}(0) &= (2\alpha^3 \mu^2 \omega^2 - 2\alpha^3 k_s \mu^2 - 2\alpha \omega^2 + 2\alpha k_s) c_0 + (3\alpha^2 \mu^2 \omega^2 \\
&- 3\alpha^2 k_s \mu^2 + \omega^2 - k_s) c_1 + 2\alpha^3 c_2 + (-\mu^2 \omega^2 + k_s \mu^2 + 3\alpha^2) c_3 \\
&+ (2\alpha^3 k_s \mu^2 - 2\alpha k_s) d_0 + (3\alpha^2 k_s \mu^2 + k_s) d_1 - k_s \mu^2 d_3, \\
\frac{d^5 w_2}{d\xi^5}(0) &= (2\alpha^3 \mu^2 \omega^2 - 2\alpha^3 k_s \mu^2 - 2\alpha \omega^2 + 2\alpha k_s) d_0 + (3\alpha^2 \mu^2 \omega^2 \\
&- 3\alpha^2 k_s \mu^2 + \omega^2 - k_s) d_1 + 2\alpha^3 d_2 + (-\mu^2 \omega^2 + k_s \mu^2 + 3\alpha^2) d_3 \\
&+ (2\alpha^3 k_s \mu^2 - 2\alpha k_s) c_0 + (3\alpha^2 k_s \mu^2 + k_s) c_1 - k_s \mu^2 c_3.
\end{aligned} \tag{4.8}$$

Using the equations (4.7, 4.8), one can present an approximate solution according to the Maclaurin series as follows

$$\begin{aligned}
w_1(\xi) &= c_0 + c_1 \xi + \frac{1}{2} c_2 \xi^2 + \frac{1}{6} c_3 \xi^3 \\
&+ \frac{1}{24} (-\alpha^2 \mu^2 \omega^2 c_0 + \alpha^2 k_s \mu^2 c_0 - \alpha^2 k_s \mu^2 d_0 + \dots) \xi^4 \\
&+ \frac{1}{120} (2\alpha^3 \mu^2 \omega^2 c_0 - 2\alpha^3 k_s \mu^2 c_0 + 2\alpha^3 k_s \mu^2 d_0 + \dots) \xi^5 + \dots, \\
w_2(\xi) &= d_0 + d_1 \xi + \frac{1}{2} d_2 \xi^2 + \frac{1}{6} d_3 \xi^3 \\
&+ \frac{1}{24} (-\alpha^2 \mu^2 \omega^2 d_0 + \alpha^2 k_s \mu^2 d_0 - \alpha^2 k_s \mu^2 c_0 + \dots) \xi^4 \\
&+ \frac{1}{120} (2\alpha^3 \mu^2 \omega^2 d_0 - 2\alpha^3 k_s \mu^2 d_0 + 2\alpha^3 k_s \mu^2 c_0 + \dots) \xi^5 + \dots
\end{aligned} \tag{4.9}$$

The equations (4.9) include eight constants ($c_0, \dots, c_3, d_0, \dots, d_3$). In order to eliminate these constants and find the natural frequency, one can use the boundary conditions (2.13-2.16). Let us consider boundary conditions for the simply supported double nanobeam. Now, the equations (4.9) can be transformed into matrix form as follows

$$\begin{bmatrix} 1 & \dots & 0 \\ \dots & \dots & \dots \\ \frac{1}{2} k_s - \frac{1}{3} \alpha k_s + \dots & \dots & 1 + \frac{1}{6} k_s \mu^2 - \dots \end{bmatrix} \begin{bmatrix} c_0 \\ \dots \\ d_3 \end{bmatrix} = 0. \tag{4.10}$$

Solving the equation (4.10), one can find the natural frequency of the double nanobeam.

4.2.1. Cracked double nanobeams

A crack divides the double nanobeam into two double nanobeams. Therefore, the crack divides the double nanobeam into four segments. Four series solutions are needed to study the cracked double nanobeam. There are sixteen constants in the four series. Additional conditions are

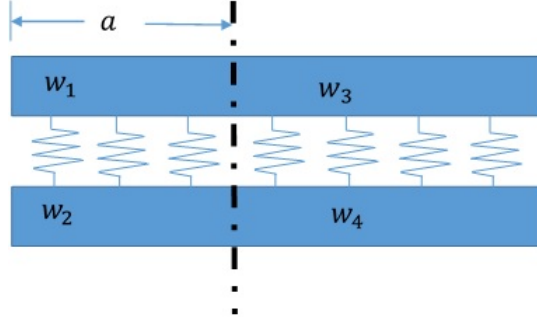


Figure 7. Modeling of the crack

for upper layer

$$w'_1 + K_1 w''_1 - w'_3 = 0, \quad (4.11)$$

and for lower layer

$$w'_2 + K_2 w''_2 - w'_4 = 0. \quad (4.12)$$

If a single crack is at the upper beam then crack severity is needed to consider K for the upper beam and the lower beam crack severity will be zero. A similar condition is applicable when the crack is only at the lower beam. When cracks are at both upper and lower beams then crack severities K_1 , K_2 are needed to consider for the upper and lower beams, respectively.

4.3. Results and discussion

In this section, numerical analysis is performed to express the effects of some parameters on the dynamic behavior of cracked tapered double nanobeam systems using tables and graphs. Here, different boundary conditions such as simply supported (SS), fully clamped (CC), clamped simply (CS), and clamped free (CF) are considered.

4.3.1. Validation and verification

In this section, obtained results are verified by comparing the results with the results of other researchers in the literature. Table 3 describes the natural frequency

of the simply supported double nanobeam where beams are considered as identical beams having rectangular cross-sections. It is seen from this table that the present results are closely matched with the results of the reference paper (Sari et al. [69]).

Table 3. Verification of frequency for various values of the spring constant and nonlocal parameter

k_s	μ	References	ω_1	ω_2	ω_3
100	0.2	Sari et al. [69]	8.3569	16.4267	24.5823
		Present	8.3569	16.4267	24.5839
	0.4	Sari et al. [69]	6.1456	14.5951	15.4197
		Present	6.1459	14.5947	15.4189
500	0.2	Sari et al. [69]	8.3569	24.5823	32.7084
		Present	8.3573	24.5839	32.7089
	0.4	Sari et al. [69]	6.1456	14.5951	22.7743
		Present	6.1469	14.5888	22.2324

Table 4. Frequency of the simply supported double nanobeam for various values of the taper ratio, nonlocal parameter and crack severity

α	μ	K	ω_1	ω_2	ω_3	ω_4	ω_5	ω_6
2	0	0	9.4868	22.1347	39.8554	44.5898	89.4140	91.6015
		0.3	8.7846	21.8222	35.5332	43.1601	85.6328	90.7578
	0.2	0	8.1079	21.5800	24.9785	31.9980	41.9726	46.2382
		0.3	7.4868	20.3457	22.8457	30.0605	40.2851	45.3164
0	0	0	9.8701	22.3027	39.4804	44.2539	88.8203	91.0546
		0.3	9.2837	22.0214	34.5019	42.7460	83.7734	90.1484
	0.2	0	8.3569	21.6738	24.5839	31.6894	41.6211	46.1757
		0.3	7.8217	20.0761	22.5332	29.7129	39.7929	45.1758

4.3.2. Effects of crack locations

The location of the crack is very significant as like as crack severity. In a simply supported beam crack at the support does not influence the natural frequency. Figures 8 and 9 describe the effect of crack location on the natural frequency for the simply supported nanobeam. In this section, nonlocal parameter, spring constant, and taper ratio are considered $\mu = 0.1$, $k_s = 200$, $\alpha = 0$, respectively. At some specific locations, an increase in crack severity decreases the natural frequency.

4.3.3. Mode shape analysis

Mode shape is a significant measure to predict the dynamic behavior of structural elements. Basically, the mode shape describes the transverse displacement from

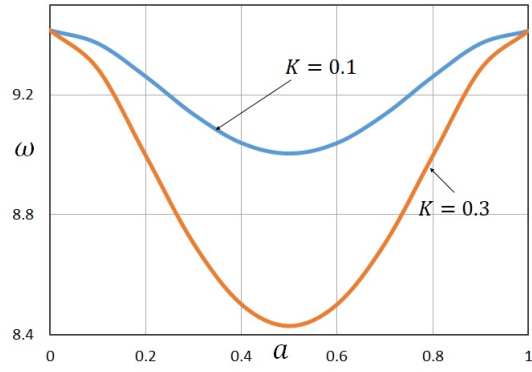


Figure 8. Frequency versus crack location with different values of crack severity (SS, first mode)

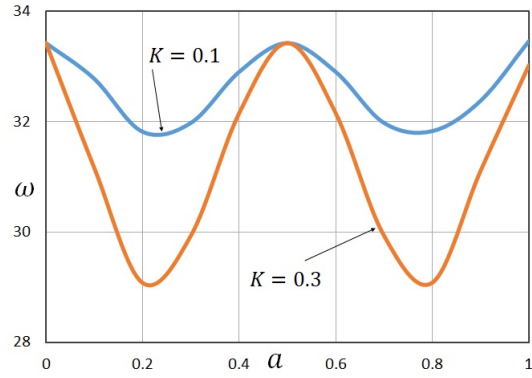


Figure 9. Frequency versus crack location with different values of crack severity (SS, second mode)

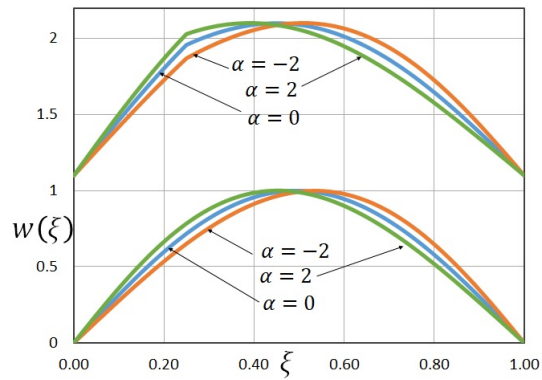


Figure 10. Mode shape of the double nanobeam for different values of taper ratio (first mode, SS)

Table 5. Frequency of the double tapered nanobeam for different values of taper ratio and boundary conditions

BC	$\alpha = 2$			$\alpha = 0$		
	ω_1	ω_2	ω_3	ω_1	ω_2	ω_3
SS SS	9.0767	16.8037	33.8652	9.4155	16.9893	33.4277
CC CC	21.7480	25.9433	51.7539	21.1074	25.4082	50.9805
CC SS	12.6416	24.3926	35.2363	12.7959	23.8808	34.8027
CF CF	1.9510	14.2764	17.7314	3.5315	14.5771	20.6777
CS CS	12.6611	18.9814	40.7461	14.5986	20.3262	41.7929

Table 6. Frequency of the simply supported double nanobeam for various values of the spring constant and crack severity

K	k_s	ω_1	ω_2	ω_3	ω_4	ω_5	ω_6
0	50	9.3306	13.6767	33.5371	34.9941	64.7695	65.5586
	250	9.3306	24.2285	33.5371	40.3086	64.7773	68.5273
	2000	9.3306	33.5371	63.9804	64.6914	71.6523	89.8672
0.3	50	8.6177	13.2471	28.4707	34.3574	60.8086	65.1914
	250	8.7075	23.7558	29.8223	38.5176	61.6679	67.3164
	2000	8.7827	30.5293	60.9258	63.9648	69.8008	87.9766

the neutral axis during vibration. Figures 10, and 11 illustrate the mode shapes for simply supported nanobeams with various values of taper ratio. A crack is considered only at the upper beam and the lower beam is fully intact. Crack severity $K = 0.3$, crack location $a = 0.25$, spring constant $k_s = 200$, nonlocal parameter $\mu = 0.1$ are considered, respectively. It is seen from these figures that mode shapes are influenced by the taper ratio. This influence increases at a higher mode of frequency. A crack at the upper beam affects the mode shape of the upper beam as well as the lower beam significantly.

4.3.4. Parametric effects

Parametric analysis is essential to comprehend the dynamic behavior of the double nanobeam properly. Natural frequency is influenced by the spring constant, nonlocal parameter, taper ratio, crack severity, and boundary conditions significantly. Table 4 describes the frequency of the simply supported double nanobeam for several values of the taper ratio, nonlocal parameter, and crack severity. A crack is considered at the location of $a = 0.25$ and spring constant $k_s = 200$ is considered. In a simply supported beam, frequency is higher in the rectangular beam than in the tapered beam. Table 5 illustrates the frequency of the double tapered nanobeam for different boundary conditions and taper ratios. The beam is considered intact where $K = 0$ and $\mu = 0.1$. In clamped beam, frequency is higher in the tapered beam than in the rectangular beam. Table 6 represents the

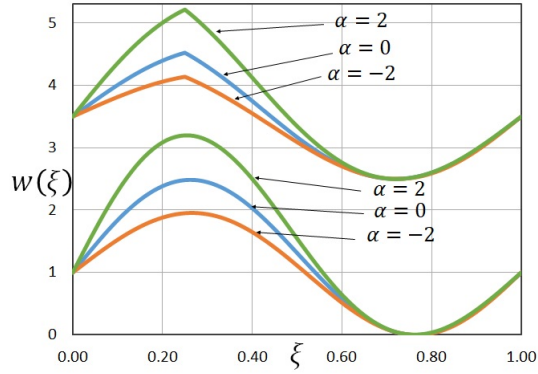


Figure 11. Mode shape of the double nanobeam for different values of taper ratio (second mode, SS)

frequency of the simply supported double nanobeam for various spring constants and crack severities where $\alpha = 1$, $\mu = 0.1$, and $a = 0.25$ are considered. Frequency increases for the increase of the spring constant. Whereas, the first mode of frequency is less effective than the higher mode of frequency for the increase of the spring constant.

5. TRANSVERSE VIBRATION OF TAPERED NANOBEAMS WITH ELASTIC SUPPORTS

A solution technique based on the homotopy perturbation method is proposed to analyze the dynamic behavior of the tapered nanobeam with elastic supports (see Hossain and Lellep [32]).

5.1. Problem description

A schematic shape of a linearly elastic, isotropic, tapered nanobeam subjected to transverse vibration is illustrated in Figure 12.

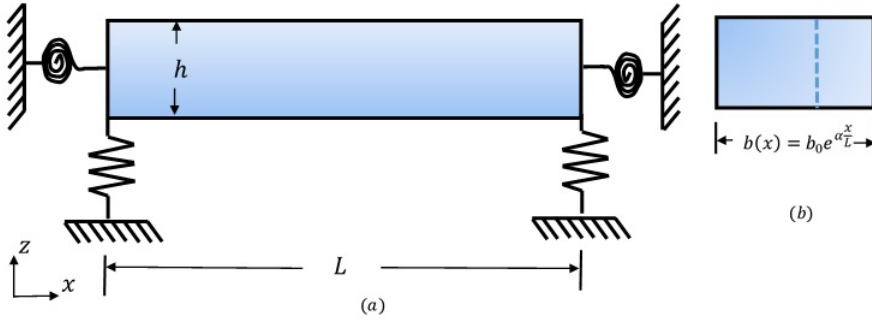


Figure 12. (a) A tapered nanobeam with elastic supports, (b) Tapered cross-section

Let the origin of the coordinate system be placed at the left end of the nanobeam. The beam axis coincides with the x -axis and the height of the beam is parallel to the z -axis. The nanobeam with length L and variable-width $b(x)$ are considered. The modulus of elasticity E and the density of the beam ρ are uniform throughout the beam. Translational and rotational flexible supports are used at both ends of the beam. Our intention is to find the effects of the taper ratio and the nonlocal parameter on the natural vibration of nanobeams.

5.2. Mathematical model

Combining the equations (2.7), and (2.10) and eliminating axial force N , the bending moment M for the nonlocal beam model can be expressed as

$$M = -(e_0 a)^2 (-\rho A(x) \frac{\partial^2 W}{\partial t^2}) - EI(x) \frac{\partial^2 W}{\partial x^2}. \quad (5.1)$$

Similarly, from the equation (5.1), the shear force Q for the nonlocal beam model can be presented as

$$Q = -(e_0 a)^2 \frac{\partial}{\partial x} (-\rho A(x) \frac{\partial^2 W}{\partial t^2}) - \frac{\partial}{\partial x} (EI(x) \frac{\partial^2 W}{\partial x^2}). \quad (5.2)$$

Combining equations (5.1), and (2.10), the governing equation of the nonlocal tapered beam can be presented as

$$\frac{\partial^2}{\partial x^2}(EI(x) \frac{\partial^2 W}{\partial x^2}) + (e_0 a)^2 \frac{\partial^2}{\partial x^2}(-\rho A(x) \frac{\partial^2 W}{\partial t^2}) + \rho A(x) \frac{\partial^2 W}{\partial t^2} = 0. \quad (5.3)$$

Substituting the values of $I(x)$ and $A(x)$ from equations (2.32), one can present (5.3) as

$$EI_0 \frac{\partial^2}{\partial x^2}(e^{\frac{\alpha x}{L}} \frac{\partial^2 W}{\partial x^2}) - (e_0 a)^2 \rho A_0 \frac{\partial^2}{\partial x^2}(e^{\frac{\alpha x}{L}} \frac{\partial^2 W}{\partial t^2}) + \rho A_0 e^{\frac{\alpha x}{L}} \frac{\partial^2 W}{\partial t^2} = 0. \quad (5.4)$$

Applying the dimensionless parameters (2.12) and transformation function (2.11), equation (5.4) can be written as

$$\frac{d^2}{d\xi^2}(e^{\alpha\xi} \frac{d^2 w}{d\xi^2}) + \mu^2 \omega^2 \frac{d^2}{d\xi^2}(e^{\alpha\xi} w) - \omega^2 e^{\alpha\xi} w = 0. \quad (5.5)$$

Simplifying the equation (5.5), one can rewrite it as follows

$$\frac{d^4 w}{d\xi^4} + 2\alpha \frac{d^3 w}{d\xi^3} + (\alpha^2 + \mu^2 \omega^2) \frac{d^2 w}{d\xi^2} + 2\alpha \mu^2 \omega^2 \frac{dw}{d\xi} + (\mu^2 \omega^2 \alpha^2 - \omega^2) w = 0. \quad (5.6)$$

Equation (5.6) is the simplified form of the governing equation. It is difficult to find the exact solution of this equation. However, it can be solved by the approximation technique.

5.2.1. Elastic boundary conditions

Elastic boundaries are modeled with the help of the flexible springs. In this paper, two types of spring boundary conditions are used. These are the rotational spring and translational spring. Rotational spring resists the moment at the boundary. On the other hand, translational spring resists shear at the boundary. The boundary conditions for the flexible supports are

$$T_0 W = Q, \quad R_0 \frac{\partial W}{\partial x} = -M, \quad \text{at } x = 0, \quad (5.7)$$

and

$$T_L W = -Q, \quad R_L \frac{\partial W}{\partial x} = M, \quad \text{at } x = L, \quad (5.8)$$

where T_0 , T_L represent spring constants for translational spring and R_0 , R_L represent spring constants for rotational spring at the point $x = 0$ and $x = L$, respectively. For the translational spring, from equations (5.7), and (5.2), it can be written as

$$T_0 W = (e_0 a)^2 \frac{\partial}{\partial x}(\rho A(x) \frac{\partial^2 W}{\partial t^2}) - \frac{\partial}{\partial x}(EI(x) \frac{\partial^2 W}{\partial x^2}). \quad (5.9)$$

Using the transformation (2.11) and dimensionless variable (2.12), the equation (5.9) can be written as

$$\frac{T_0 L^3}{EI_0} w = -\mu^2 \omega^2 e^{\alpha \xi} \left(\frac{\partial w}{\partial \xi} + \alpha w \right) - e^{\alpha \xi} \left(\frac{\partial^3 w}{\partial \xi^3} + \alpha \frac{\partial^2 w}{\partial \xi^2} \right). \quad (5.10)$$

The equation (5.10) can be presented as follows

$$t_0 w = -\mu^2 \omega^2 e^{\alpha \xi} \left(\frac{\partial w}{\partial \xi} + \alpha w \right) - e^{\alpha \xi} \left(\frac{\partial^3 w}{\partial \xi^3} + \alpha \frac{\partial^2 w}{\partial \xi^2} \right). \quad (5.11)$$

where $t_0 = \frac{T_0 L^3}{EI_0}$ is the dimensionless translational spring constant. If the origin of coordinates is the left boundary of the beam, the boundary condition can be written as follows

$$t_0 w = -\mu^2 \omega^2 \left(\frac{\partial w}{\partial \xi} + \alpha w \right) - \left(\frac{\partial^3 w}{\partial \xi^3} + \alpha \frac{\partial^2 w}{\partial \xi^2} \right). \quad (5.12)$$

Similarly for the rotational spring, according to the equations (5.7), and (5.1), it can be written as

$$R_0 \frac{\partial W}{\partial x} = -(e_0 a)^2 \rho A(x) \frac{\partial^2 W}{\partial t^2} + EI(x) \frac{\partial^2 W}{\partial x^2}. \quad (5.13)$$

Using the transformation (2.11) and dimensionless variable (2.12), the equation (5.13) can be simplified as

$$r_0 \frac{\partial w}{\partial \xi} = e^{\alpha \xi} \mu^2 \omega^2 w + e^{\alpha \xi} \frac{\partial^2 w}{\partial \xi^2}. \quad (5.14)$$

where $r_0 = \frac{R_0 L}{EI_0}$ is the dimensionless rotational spring constant. At the left corner of the beam, where $\xi = 0$, the equation (5.14) can be written as

$$r_0 \frac{\partial w}{\partial \xi} = \mu^2 \omega^2 w + \frac{\partial^2 w}{\partial \xi^2}. \quad (5.15)$$

In a similar way, from the equation (5.15), one can find the other two boundary conditions at $\xi = 1$ for translational and rotational springs as follows

$$t_L w = \mu^2 \omega^2 e^{\alpha} \left(\alpha w + \frac{\partial w}{\partial \xi} \right) + e^{\alpha} \left(\alpha \frac{\partial^2 w}{\partial \xi^2} + \frac{\partial^3 w}{\partial \xi^3} \right), \quad (5.16)$$

and

$$r_L \frac{\partial w}{\partial \xi} = -\mu^2 \omega^2 e^{\alpha} w + e^{\alpha} \frac{\partial^2 w}{\partial \xi^2}. \quad (5.17)$$

The required four boundary conditions for the connected springs at both ends of the beam can be presented as
at $\xi = 0$,

$$\begin{aligned} t_0 w + \mu^2 \omega^2 \left(\frac{\partial w}{\partial \xi} + \alpha w \right) + \left(\frac{\partial^3 w}{\partial \xi^3} + \alpha \frac{\partial^2 w}{\partial \xi^2} \right) &= 0, \\ r_0 \frac{\partial w}{\partial \xi} - \mu^2 \omega^2 w - \frac{\partial^2 w}{\partial \xi^2} &= 0, \end{aligned} \quad (5.18)$$

and at $\xi = 1$,

$$\begin{aligned} t_L w - \mu^2 \omega^2 e^\alpha \left(\alpha w + \frac{\partial w}{\partial \xi} \right) - e^\alpha \left(\alpha \frac{\partial^2 w}{\partial \xi^2} + \frac{\partial^3 w}{\partial \xi^3} \right) &= 0, \\ r_L \frac{\partial w}{\partial \xi} + \mu^2 \omega^2 e^\alpha w + e^\alpha \frac{\partial^2 w}{\partial \xi^2} &= 0. \end{aligned} \quad (5.19)$$

5.3. Solution of the problem

This problem can be solved using the homotopy perturbation method. Applying the basic rules of the homotopy perturbation method (2.39), the governing equation (5.6) of the problem can be written as

$$\begin{aligned} \frac{d^4 w}{d\xi^4} + P[2\alpha \frac{d^3 w}{d\xi^3} + (\alpha^2 + \mu^2 \omega^2) \frac{d^2 w}{d\xi^2} \\ + 2\alpha \mu^2 \omega^2 \frac{dw}{d\xi} + \omega^2 (\mu^2 \alpha^2 - 1)w] &= 0. \end{aligned} \quad (5.20)$$

Considering the solution (2.40) of the equation (5.20), the coefficient of the power of P can be written as

$$\begin{aligned} P^0 &\rightarrow \frac{d^4}{d\xi^4} w_0(\xi) = 0, \\ P^1 &\rightarrow \frac{d^4}{d\xi^4} w_1(\xi) + 2\alpha \left(\frac{d^3}{d\xi^3} w_0(\xi) \right) + (\xi^2 + \mu^2 \omega^2) \left(\frac{d^2}{d\xi^2} w_0(\xi) \right) \\ &\quad + 2\alpha \mu^2 \omega^2 \left(\frac{d}{d\xi} w_0(\xi) \right) + (\mu^2 \omega^2 \alpha^2 - \omega^2) w_0 = 0, \\ P^2 &\rightarrow \frac{d^4}{d\xi^4} w_2(\xi) + 2\alpha \left(\frac{d^3}{d\xi^3} w_1(\xi) \right) + (\alpha^2 + \mu^2 \omega^2) \left(\frac{d^2}{d\xi^2} w_1(\xi) \right) \\ &\quad + 2\alpha \mu^2 \omega^2 \left(\frac{d}{d\xi} w_1(\xi) \right) + (\mu^2 \omega^2 \alpha^2 - \omega^2) w_1 = 0. \end{aligned} \quad (5.21)$$

The solution of the equation (5.21) is presented as follows

$$w_0 = A_0 + A_1 \xi + A_2 \frac{\xi^2}{2} + A_3 \frac{\xi^3}{6}. \quad (5.22)$$

The remaining terms of the series can be calculated as

$$w_i = -2\alpha L^{-1}\left(\frac{d^3}{d\xi^3}w_{i-1}\right) - (\alpha^2 + \mu^2\omega^2)L^{-1}\left(\frac{d^2}{d\xi^2}w_{i-1}\right) - 2\alpha\mu^2\omega^2L^{-1}\left(\frac{d}{d\xi}w_{i-1}\right) - (\mu^2\omega^2\alpha^2 - \omega^2)L^{-1}(w_{i-1}), \quad (5.23)$$

where $i = 1, 2, 3, \dots, n$ and $L^{-1} = \int \int \int \int d\xi d\xi d\xi d\xi$. Using the equation (5.23), one can calculate the following terms as below

$$w_1 = \left(-\frac{1}{24}\alpha^2\mu^2\omega^2\xi^4 + \frac{1}{24}\omega^2\xi^4\right)A_0 + \left(-\frac{1}{12}\mu^2\omega^2\alpha\xi^4 - \frac{1}{120}\alpha^2\mu^2\omega^2\xi^5 + \frac{1}{120}\omega^2\xi^5\right)A_1 + \left(-\frac{1}{24}\mu^2\omega^2\xi^4 - \frac{1}{24}\alpha^2\xi^4 - \frac{1}{60}\mu^2\omega^2\alpha\xi^5 - \frac{1}{720}\alpha^2\mu^2\omega^2\xi^6 + \frac{1}{720}\omega^2\xi^6\right)A_2 + \left(-\frac{1}{12}\alpha\xi^4 - \frac{1}{120}\mu^2\omega^2\xi^5 - \frac{1}{120}\alpha^2\xi^5 - \frac{1}{360}\mu^2\omega^2\alpha\xi^6 - \frac{1}{5040}\alpha^2\mu^2\omega^2\xi^7 + \frac{1}{5040}\omega^2\xi^7\right)A_3, \quad (5.24)$$

and

$$w_2 = \left(-\frac{1}{60}\alpha\omega^2\xi^5 - \frac{1}{720}\mu^2\omega^2\xi^6 - \frac{1}{720}\alpha^2\omega^2\xi^6 + \dots\right)A_0 + \left(-\frac{1}{360}\alpha\omega^2\xi^6 - \frac{1}{5040}\mu^2\omega^2\xi^7 - \frac{1}{5040}\alpha^2\omega^2\xi^7 + \dots\right)A_1 + \left(-\frac{1}{2520}\alpha\omega^2\xi^7 + \frac{1}{720}\mu^4\omega^4\xi^6 - \frac{1}{20160}\mu^2\omega^4\xi^8 + \dots\right)A_2 + \left(-\frac{1}{10080}\alpha\omega^2\xi^8 + \frac{1}{5040}\mu^4\omega^4\xi^7 - \frac{1}{181440}\mu^2\omega^4\xi^9 + \dots\right)A_3. \quad (5.25)$$

Here, w_0, w_1, w_2 present three terms of the series. The accuracy of this calculation depends on the number of terms. For achieving good accuracy, 40 terms of this series have been calculated. Applying the boundary conditions (5.18, 5.19) or any set of (2.13-2.16), w series can be presented in matrix form as below

$$[M(\omega)][A] = 0. \quad (5.26)$$

The natural frequency of the nanobeam is calculated from the equation (5.26).

5.4. Results and discussion

In this section, the efficiency of the homotopy perturbation method for solving the transverse vibration problems of tapered nanobeams with elastic supports is demonstrated. This study can be described by the following steps. First of all, the results of the present paper are examined and compared with the results of

published papers in the literature. Secondly, frequency in different modes of the presented model is illustrated in tabular form for rigid boundary supports. Thirdly, frequency ratio versus taper ratio for different supports are presented graphically. Finally, the mode shapes of the nanobeam are presented for different values of the taper ratio.

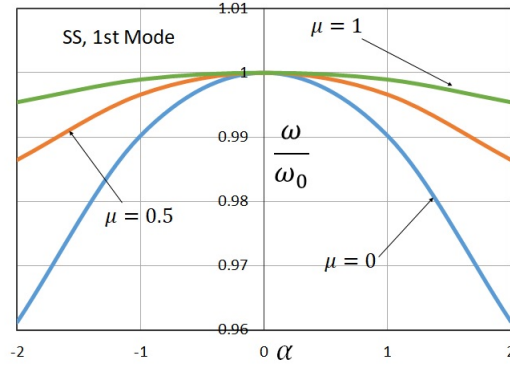


Figure 13. Frequency ratio versus taper ratio (first mode, SS)

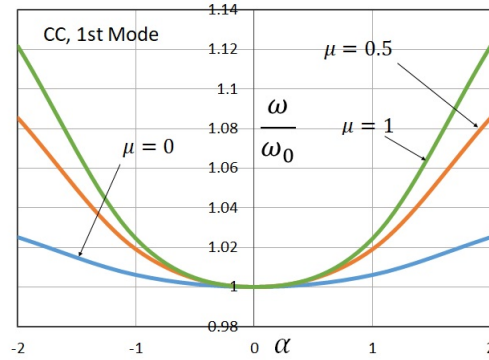


Figure 14. Frequency ratio versus taper ratio (first mode, CC)

In Table 7, obtained frequencies of the present tapered nanobeam are also compared with the frequencies of a regular nanobeam. For this comparison, taper ratio $\alpha = 0$ is considered in the present study. Obtained results show good agreement with the results of Phadikar and Pradhan [60]. On the other hand, in Table 8, the natural frequencies of tapered nanobeams obtained in the present study are compared with the natural frequencies of the tapered macro beam. For this comparison, the nonlocal parameter $\mu = 0$ is considered. The results obtained in the present study show very good agreement with the results of Mao and Pietrzko [56]. Table 9 describes the natural frequency of tapered nanobeams with different rigid supports. It is clear from this table that frequency decreases with the increase of the nonlocal parameter. On the other hand, the effect of taper ratio on the frequency depends on the end support system.

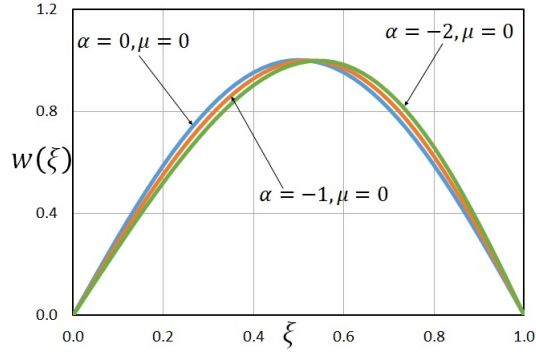


Figure 15. Mode shape for different taper ratios (SS, first mode)

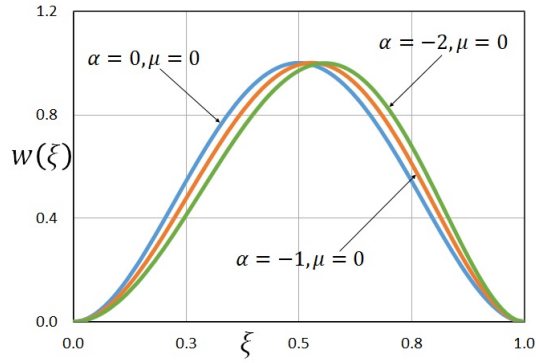


Figure 16. Mode shape for different taper ratios (CC, first mode)

Table 7. Data comparison with the regular (uniform) nanobeam

Boundary	Method	δ	μ	frequency (ω)		
				1	2	3
SS	[60]	0	1	2.9936	6.2061	9.3796
	Present	0	1	2.9934	6.2055	9.3725
CC	[60]	0	1	6.0574	8.9011	12.4822
	Present	0	1	6.0561	8.8950	12.4521

Table 8. Data comparison with the macro tapered beam

Boundary	Method	δ	μ	frequency (ω)			
SS				1	2	3	4
	[56]	0	0	9.86960	39.47841	88.82643	157.9136
	Present	0	0	9.87011	39.48046	88.82031	157.9218
	[56]	-1	0	9.77291	39.57036	88.97052	158.0841
	Present	-1	0	9.77246	39.57421	88.97656	158.0781
	[56]	-2	0	9.48725	39.85231	89.40520	158.5968
	Present	-2	0	9.48681	39.85546	89.39843	158.6093

Table 9. Natural frequency of the tapered nanobeam with different rigid supports

		SS			CC		
μ	α	1	2	3	1	2	3
0	0	9.87011	39.4804	88.8203	22.3722	61.6718	120.898
1	0	2.99340	6.20556	9.37255	6.05615	8.89501	12.4521
2	0	1.55126	3.13183	4.70585	3.11230	4.48164	6.26835
0	-1	9.77324	39.5675	88.9765	22.5103	61.8607	121.109
1	-1	2.99013	6.21035	9.37421	6.20361	8.79931	12.6028
2	-1	1.55087	3.13261	4.70585	3.19345	4.42382	6.37382
0	-2	9.48671	39.8530	89.3984	22.9366	62.4199	121.725
1	-2	2.97988	6.22773	9.37734	6.79335	8.34355	12.8320
2	-2	1.54931	3.13496	4.70664	3.55449	4.12416	6.48007

Figures 13, and 14 illustrate frequency ratio versus taper ratio for different values of the nonlocal parameter at simply supported and clamped nanobeams. The frequency ratio is calculated from the frequency of the tapered beam and the frequency of the regular beam for three different values of the nonlocal parameter. The frequency ratio shows different patterns for different support systems. It is clear that the sign of taper ratio is not effective in the case of simply supported and clamped nanobeams.

Figures 15, and 16 illustrate the shape mode of simply supported and fully clamped tapered nanobeams, respectively. It is clear from these figures that the increase in the value of the taper ratio increases the variation of the mode shape.

6. NATURAL VIBRATION OF AXIALLY GRADED MULTI CRACKED NANOBEAMS IN THERMAL ENVIRONMENT USING POWER SERIES

In this section, a numerical investigation of the dynamic characteristics of axially graded multi cracked nanobeams in a thermal environment is demonstrated by applying the power series solution technique (see Hossain and Lellep [36]).

6.1. Description of the problem

The geometry of an axially graded multi-cracked nanobeam is illustrated in Figure 17. The left endpoint of the beam is located at the origin of the coordinate system. The neutral axis of the beam overlaps with the x-axis and the height of the beam is placed along the z-axis. Here, L , b , h describe the length, width and height of the beam, respectively. Multiple open cracks are considered at the distance of a_i to a_n from the left endpoint. The material of the beam is axially graded along the x-axis. E and ρ are the elasticity and density varying exponentially from one end to another end. The objective of this analysis is to examine the natural vibration of the axially graded multi cracked nanobeam.

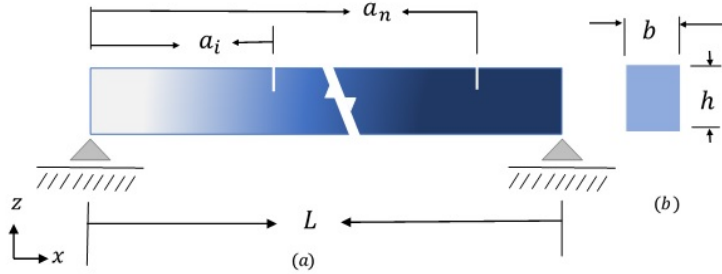


Figure 17. (a) An axially graded multi-cracked nanobeam, (b) Cross-section of the beam

6.2. Mathematical model

6.2.1. Derivation of the governing equation

In this paper, the beam is axially graded and also subjected to thermal load. Therefore, the material properties such as elasticity and density (2.33) are varying along the x-axis. Similarly, the thermal load (2.34) is also varying with the elasticity as an axial load. Considering these variations, and combination of equations (2.7)

and (2.10) can be expressed as

$$\begin{aligned} \frac{\partial^2}{\partial x^2} (E(x)I \frac{\partial^2 W}{\partial x^2}) - (e_0 a)^2 \frac{\partial^2}{\partial x^2} [\rho(x)A \frac{\partial^2 W}{\partial t^2} - \frac{\partial}{\partial x} (N(x) \frac{\partial W}{\partial x})] \\ - \frac{\partial}{\partial x} (N(x) \frac{\partial W}{\partial x}) + \rho(x)A \frac{\partial^2 W}{\partial t^2} = 0. \end{aligned} \quad (6.1)$$

Using the transformation (2.11) and non-dimensional parameters (2.12) and applying the axially graded material properties (2.33), the equation (6.1) can be expressed as

$$\begin{aligned} (\lambda^2 \frac{d^2 w}{d\xi^2} + 2\lambda \frac{d^3 w}{d\xi^3} + \frac{d^4 w}{d\xi^4}) + \mu^2 \omega^2 (\lambda^2 w + 2\lambda \frac{dw}{d\xi} + \frac{d^2 w}{d\xi^2}) \\ - \mu^2 n (\lambda^3 \frac{dw}{d\xi} + 3\lambda^2 \frac{d^2 w}{d\xi^2} + 3\lambda \frac{d^3 w}{d\xi^3} + \frac{d^4 w}{d\xi^4}) + n (\frac{dw}{d\xi} + \frac{d^2 w}{d\xi^2}) - \omega^2 w = 0, \end{aligned} \quad (6.2)$$

here, λ is the parameter of non-homogeneity, n indicates the dimensionless thermal load. Therefore, the equation (6.2) can be presented in a simplified form as

$$\frac{d^4 w}{d\xi^4} + \beta_1 \frac{d^3 w}{d\xi^3} + \beta_2 \frac{d^2 w}{d\xi^2} + \beta_3 \frac{dw}{d\xi} + \beta_4 w = 0, \quad (6.3)$$

where

$$\begin{aligned} \beta_1 = \frac{2\lambda - 2\lambda\mu^2 n}{1 - \mu^2 n}, \quad \beta_2 = \frac{\lambda^2 + \mu^2 \omega^2 - 3\mu^2 n \lambda^2 + n}{1 - \mu^2 n}, \\ \beta_3 = \frac{2\mu^2 \omega^2 \lambda - \mu^2 n \lambda^3 + n}{1 - \mu^2 n}, \quad \beta_4 = \frac{\mu^2 \omega^2 \lambda^2 - \omega^2}{1 - \mu^2 n}. \end{aligned}$$

In this nanobeam, multiple cracks are considered where n number of cracks separate the beam into $n + 1$ segments. Finally, the governing equations for $n + 1$ number of segments can be expressed as

$$\frac{d^4 w_i}{d\xi^4} + \beta_1 \frac{d^3 w_i}{d\xi^3} + \beta_2 \frac{d^2 w_i}{d\xi^2} + \beta_3 \frac{dw_i}{d\xi} + \beta_4 w_i = 0, \quad (6.4)$$

where $i = 1, 2, 3, \dots, n + 1$. These equations (6.4) represent the set of governing equations considering the $n + 1$ segments that are separated at the location of cracks a_j , where $j = 1, 2, \dots, n$. One can solve these equations (6.4) by applying in-between conditions (2.31) at the crack locations and any one set of end conditions (2.13-2.16) for the boundary supports.

6.3. Power series solution

The power series solution technique is very useful to solve linear and nonlinear problems. The governing equation can be solved using the power series solution technique. Using the rules of power series (2.48-2.50), one can replace the

derivatives of deflection in the governing equation (6.4). Therefore, simplifying the equation can be written as

$$A_{k+4} = -\frac{\beta_1}{(k+4)}A_{k+3} - \frac{\beta_2}{(k+4)(k+3)}A_{k+2} - \frac{\beta_3}{(k+4)(k+3)(k+2)}A_{k+1} - \frac{\beta_4}{(k+4)(k+3)(k+2)(k+1)}A_k, \quad (6.5)$$

here, $A_{k+4}, A_{k+3}, \dots, A_k$ are the power series coefficients. Using the above relation in the equation (6.5), one can calculate the series coefficients as follows

$$\begin{aligned} A_4 &= -\frac{1}{4}\beta_1 A_3 - \frac{1}{12}\beta_2 A_2 - \frac{1}{24}\beta_3 A_1 - \frac{1}{24}\beta_4 A_0, \\ A_5 &= \frac{1}{20}(\beta_1^2 - \beta_2)A_3 + \frac{1}{60}(\beta_1\beta_2 - \beta_3)A_2 \\ &\quad + \frac{1}{120}(\beta_1\beta_3 - \beta_4)A_1 + \frac{1}{120}\beta_1\beta_4 A_0, \\ A_6 &= -\frac{1}{120}(\beta_1^3 - 2\beta_1\beta_2 + \beta_3)A_3 - \frac{1}{360}(\beta_1^2\beta_2 - \beta_1\beta_3 - \beta_2^2 + \beta_4)A_2 \\ &\quad - \frac{1}{720}(\beta_1^2\beta_3 - \beta_1\beta_4 - \beta_2\beta_3)A_1 - \frac{1}{720}(\beta_1^2\beta_4 - \beta_2\beta_4)A_0. \end{aligned} \quad (6.6)$$

Applying the series coefficients, the solution for each segment of the beam can be written as

$$w_i = A_{i,0} + A_{i,1}\xi + A_{i,2}\xi^2 + A_{i,3}\xi^3 + \sum_{k=4}^{\infty} A_{i,k}\xi^k, \quad (6.7)$$

where $i = 1, 2, \dots, n+1$. Using the simply-supported boundary conditions (2.13) and intermediate conditions for the crack (2.31), one can eliminate constant coefficients ($A_{i,k}$) and solve these equations to form a matrix as follows

$$\begin{bmatrix} m_{1,1} & \dots & \dots & m_{1,4(n+1)} \\ \dots & \dots & \dots & \dots \\ \dots & \dots & \dots & \dots \\ m_{4(n+1),1} & \dots & \dots & m_{4(n+1),4(n+1)} \end{bmatrix} \begin{bmatrix} A_{1,0} \\ \dots \\ \dots \\ A_{(n+1),3} \end{bmatrix} = 0, \quad (6.8)$$

here, $m_{1,1}, \dots, m_{4(n+1),4(n+1)}$ indicate the elements of the matrix. The value of natural frequency for the multi-cracked nanobeam can be determined by solving the above matrix equation (6.8). In this section, single, double, and triple cracked nanobeams are analyzed.

6.4. Numerical results and discussion

In this section, parametric analysis is performed to scrutinize their impacts on the natural frequency of the axially graded multi cracked nanobeam in a thermal environment. First of all, the efficiency of the solution technique is examined by

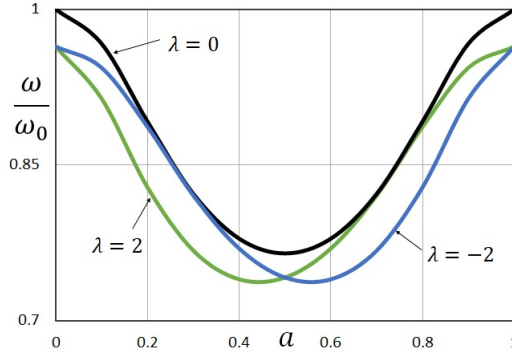


Figure 18. Frequency ratio versus crack position (SS, first mode)

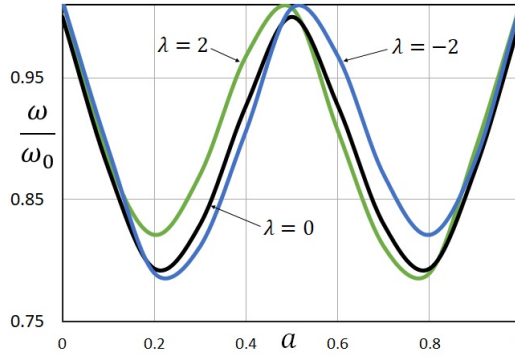


Figure 19. Frequency ratio versus crack position (SS, second mode)

comparing the obtained results with the results of relevant papers in the existing literature. In addition, the influences of crack position on the various modes of natural frequency for several values of the gradient parameter are studied and presented graphically. Moreover, the effects of temperature, non-homogeneity, and the nonlocal parameter on the natural frequency are presented in tabular forms. Finally, the mode shapes are depicted to investigate the axially graded multi cracked nanobeam and to study the effects of crack severity on the transverse deflection.

6.4.1. Comparison of results

In this section, the outcomes of the study are compared with the outcomes of some benchmark research in the existing literature. This comparison is performed by investigating the effects of nonlocal parameter (μ), thermal load (n), and crack severity on different modes of the natural frequency. In Table 10, the first mode of natural frequency (square root) for simply supported nanobeams is presented. The effect of cracks is not considered as well as the beam is considered uniform and homogeneous in this section of calculation. Different values of nonlocal parameter (μ) and thermal load (n) are considered. It is understandable from this

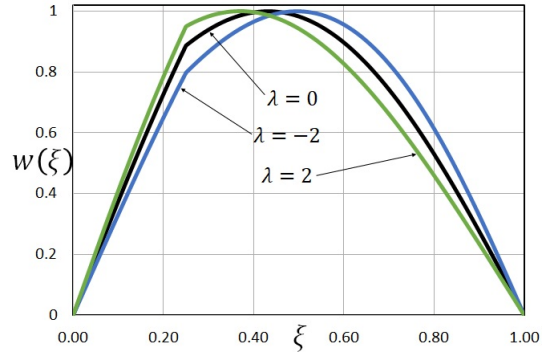


Figure 20. Mode shape for different values gradient parameter (first mode, SS)

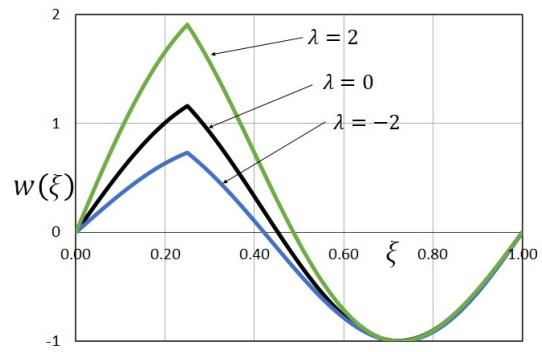


Figure 21. Mode shape for different values gradient parameter (second mode, SS)

tabular data that the natural frequency decreases with the increase of the nonlocal parameter. Therefore, the natural frequency increases for the decrease of temperature. Outcomes are verified by the results of Esen et al. [20] and Aria et al. [6]. In Table 11, different modes of natural frequency for a double cracked beam with different values of the nonlocal parameter and crack severity are studied. Two cracks in the beam are located at $a_1 = 0.3$ and $a_2 = 0.7$, respectively. It is obvious from this tabular data that frequency decreases with the increase of the nonlocal parameter. Values of frequency are also influenced by the crack severity. This table data are also compared with the results of Roostai and Haghpanahi [66]. This comparison shows a close match among these results. Similarly, in Table 12, different modes of natural frequency (square root) for the simply supported single cracked nanobeam are demonstrated. Non-homogeneity of nanobeams is ignored in this section of calculation. Different values of crack positions (a), crack severity (K), nonlocal parameter (μ) are also considered. It is comprehensible from this tabular data that the natural frequency decreases with the increase of crack severity. Frequency shows the lower value at the crack position of $a = 0.5$ than $a = 0.25$. This table data also match with the results of Esen et al. [20] and Aria et al. [6]. These evaluations show a good agreement between current data and the data of other researchers.

6.4.2. Effect of crack locations

The location of the crack is also crucial as like as the crack severity for analyzing the cracked beam. In Figures 18, 19, first modes of frequency for simply supported nanobeams are demonstrated. Thermal load is ignored in this section of the calculation. The frequency ratio is calculated by the frequency at any point with non-homogeneity and the frequency at the initial point without considering non-homogeneity. In this section, nonlocal parameter $\mu = 0.1$ and crack severity $K = 0.35$ are applied. It is evident from these figures that in the case of simply supported and the first mode of frequency, the frequency ratio slightly decreases for the presence of non-homogeneity. However, in other cases, frequency increases with the presence of non-homogeneity. It is very important in these curves that the frequency ratio is not affected by the positive or negative sign of non-homogeneity. The effect of crack location is also influenced by the end support systems.

6.4.3. Frequency for axially graded intact and cracked nanobeams under thermal load

In this section, Tables 13, and 14 represent the effect of temperature on the natural frequency for axially graded intact or cracked nanobeams, respectively. Interactions between temperature, non-homogeneity, nonlocal parameter, end supports, and different modes of frequency of intact and cracked nanobeams are very diverse. Table 13 represents the relationship between the nonlocal parameter,

Table 10. Natural frequency (square root) for varying nonlocal parameter, thermal load and different end supports

SS	$\mu = 0$			$\mu = 0.1$		
n	Present	[20]	[6]	Present	[20]	[6]
2	2.9688	2.9672	2.9680	2.8815	2.8810	2.8800
1	3.0589	3.0575	3.0563	2.9793	2.9791	2.9787
0	3.1416	3.1416	3.1415	3.0684	3.0683	3.0680
-1	3.2184	3.2173	3.2180	3.1505	3.1504	3.1501
-2	3.2901	3.2896	3.2894	3.2268	3.2266	3.2262

Table 11. Natural frequency for the double cracked beam with different values of nonlocal parameter and crack severity

		SS			
		K=0.0325		K=0.075	
μ	Mode	[66]	Present	[66]	Present
0	1	9.474	9.475	9.023	9.023
	2	37.335	37.338	34.986	34.986
	3	88.313	86.383	87.741	85.883
1	1	2.873	2.873	2.732	2.732
	2	5.858	5.858	5.46	5.459
	3	9.313	9.313	9.236	9.236

thermal load, and non-homogeneity in absence of cracks. In this section, three different values of the nonlocal parameter, thermal load from -2 to 2 and non-homogeneity -2 to 2 are considered. It is very clear that the frequency decreases for the increase of the nonlocal parameter. On the other hand, frequency increases with the decrease of temperature. Table 14 represents the relationship between the nonlocal parameter, thermal load and non-homogeneity in the presence of a crack. In this section, different values of nonlocal parameter, thermal load and non-homogeneity are considered as in Table 13. In addition, a crack is considered at crack location $a = 0.25$ with the crack severity $K = 0.35$. The relationship between thermal load, non-homogeneity, and natural frequency becomes more changeable in the presence of cracks for various modes of frequency. In Table 15, the natural frequency for a triple cracked beam with several values of the nonlocal parameter and crack severity is demonstrated. Three specific cracks are considered at the position of $a_1 = 0.3$, $a_2 = 0.5$, $a_3 = 0.7$, respectively. The effects of non-homogeneity and temperature are not considered. It is understandable in this tabular data that the frequency decreases with the increase of the value of the nonlocal parameter and crack severity.

Table 12. Natural frequency (square root) for the simply supported cracked nanobeam in different mode with varying nonlocal parameter

a	K	mode	$\mu = 0.0$			$\mu = 0.2$		
			Present	[20]	[6]	Present	[20]	[6]
0.5	0	1	3.1416	3.1413	3.1409	2.8908	2.8907	2.8907
		2	6.2833	6.2830	6.2818	4.9582	4.9580	4.9578
	0.35	1	2.7496	2.7493	2.7489	2.5232	2.5232	2.5232
		2	6.2833	6.2825	6.2819	4.9582	4.9580	4.9578
0.25	0	1	3.1416	3.1416	3.1409	2.8908	2.8908	2.8908
		2	6.2833	6.2825	6.2818	4.9582	4.9580	4.9580
	0.35	1	2.9072	2.9068	2.9064	2.6646	2.6645	2.6645
		2	5.6491	5.6486	5.6484	4.4168	4.4169	4.4168

Table 13. Frequency for several values of nonlocal parameter, thermal load and nonhomogeneity without crack

SS		$K = 0$					
n	λ	$\mu = 0$			$\mu = 0.2$		
		1	2	3	1	2	3
-2	-2	10.3271	40.7773	90.3672	8.9497	26.4121	43.9336
	0	10.8252	40.4648	89.8203	9.4653	26.1387	43.7070
	2	10.5127	40.8320	90.3984	9.1021	26.4551	43.9570
0	-2	9.48682	39.8555	89.3984	8.1079	24.9785	41.9179
	0	9.87012	39.4805	88.8203	8.3569	24.5839	41.6289
	2	9.48682	39.8555	89.3984	8.1079	24.9785	41.9179
2	-2	8.58057	38.9004	88.4297	7.1704	23.4550	39.8008
	0	8.81396	38.4668	87.8203	7.0796	22.9199	39.4336
	2	8.33545	38.8496	88.3984	6.9741	23.4082	39.7773

6.4.4. Mode shape illustration

Mode shape is one of the important characteristics that explain the vibration of structural components. It describes the transverse displacement from the beam axis during vibration. It is a significant measure that explains the pattern of vibration. In this section, the effects of non-homogeneity on the mode shape of cracked nanobeams are illustrated. In order to understand the dynamic behavior of cracked nanobeams, mode shape analysis is necessary. In Figures (20, 21), different mode shapes of nanobeams are demonstrated where the location of crack $a = 0.25L$, value of the nonlocal parameter $\mu = 0.1$, and crack severity $K = 0.35$ are considered. In this section of calculation, thermal load $n = 1$ is considered. Different values of non-homogeneity $\lambda = 2$, $\lambda = 0$ and $\lambda = -2$ are applied. It is comprehensible from these figures that mode shapes of cracked nanobeam are considerably changed by the non-homogeneity. Variation changes with the change

Table 14. Frequency for several values of nonlocal parameter, thermal load and nonhomogeneity with crack

SS $K = 0.35, a = 0.25$		$\mu = 0$			$\mu = 0.2$		
n	λ	1	2	3	1	2	3
-2	-2	9.1362	32.1699	82.4297	7.7495	20.2324	40.2070
	0	9.2583	32.7246	83.3203	7.9887	20.5723	40.1445
	2	8.6909	33.9824	85.3359	7.4907	21.3496	40.6367
0	-2	8.4341	31.4004	81.5078	7.0942	19.2588	38.4238
	0	8.4517	31.9121	82.3828	7.1001	19.5088	38.3457
	2	7.8081	33.1894	84.3984	6.6812	20.3730	38.9394
2	-2	7.6694	30.6113	80.5859	6.3442	18.2119	36.5410
	0	7.5503	31.0762	81.4297	6.0474	18.3486	36.4394
	2	6.7993	32.3809	83.4765	5.7388	19.3057	37.1426

Table 15. Natural frequency for the triple cracked nanobeam with several values of nonlocal parameter and crack severity

SS	K=0.0325			K=0.075		
μ	1	2	3	1	2	3
0	9.203	37.338	83.977	8.506	34.986	80.523
0.5	4.941	11.306	17.692	4.563	10.542	16.655
1	2.79	5.858	8.992	2.576	5.459	8.459
CC						
0	21.708	58.508	107.492	21.037	55.223	102.602
0.5	10.622	16.247	22.545	10.229	15.138	20.393
1	5.846	8.363	11.522	5.622	7.786	10.405

of the value of non-homogeneity. This variation also increases in the higher mode of frequency.

7. CONCLUDING REMARKS

In this study, the dynamic behavior of nanobeams considering various physical and geometrical properties has been analyzed. Conventional continuum theory and Eringen's nonlocal theory of elasticity have been used to simulate the problem. Several physical properties such as axially graded material and temperature have been considered. In an axially graded beam, material properties such as modulus of elasticity and density vary from one end to another end exponentially. The effect of temperature has been considered uniformly over the beam where the thermal load is accumulated with the mechanical load. Similarly, several geometrical properties such as varying cross-sections and cracks have been considered. The width of the beam is changing exponentially from one end to another end. Single and multiple cracks have been considered where cracks are simulated with the help of the rotational spring model. In this study, a double nanobeam has been analyzed where both beams are connected to each other by the Winkler-type spring model.

The most significant part of this analysis is employing several numerical techniques that are rare in the analyses of the nano-material. These techniques are the homotopy perturbation method, power series solution technique, and Maclaurin series solution technique. These techniques have been successfully applied and the obtained results have been verified with the results of existing literature.

In this study, the frequency of nanobeams decreases with the increase of crack depth. Frequency is affected by the crack location. The effect of crack location is also influenced by the end support systems. In the case of double nanobeams, natural frequency increases if the spring constant increases. The first mode of frequency is less influenced than the higher mode of frequency for the increase of spring constants. In the case of taper and non-homogeneous beams, frequency is affected by the taper ratio and non-homogeneity. The effects of taper ratio and non-homogeneity increase with the increase of the mode of frequency. However, the frequency is not affected by the sign (negative or positive) of the taper ratio and non-homogeneity in simply supported and fully clamped nanobeams. The effect of elastic supports is influenced by the nonlocal parameter and taper ratio. Nonlocal parameter reduces the stiffness of elastic supports. An increase in thermal load decreases frequency. Macrobeams and nanobeams both are similarly affected by the crack severity, taper ratio, non-homogeneity, and temperature.

In this study, a convenient way to depict the mode shape diagram of nanobeams has been presented. Therefore, the dynamic behavior of nanobeams has been illustrated with the aid of the mode shape diagram. It is evident from this analysis that the mode shape of nanobeams is significantly affected by the nonlocal parameter, taper ratio, non-homogeneity, crack severity, and the influence of temperature. Nonetheless, crack is one of the influential factors in changing the mode shape.

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Summary

In this dissertation, an analysis of the dynamic behavior of nanobeams with different physical and geometrical properties using several numerical techniques is presented. Euler-Bernoulli beam theory and nonlocal theory of elasticity are used to simulate the nanobeam.

Nanobeams are considered with some special requirements such as tapered, axially graded, and double beams. First of all, in a tapered beam, the width of the beam is varying exponentially along the x-axis from one end to another end. The properties of the tapered beam are to reduce material consumption and provide the cross-sectional area according to the moment distribution. Secondly, in an axially graded beam, material properties such as elasticity and density are varying exponentially from one end to another end. The axially graded beam can be considered as a non-homogeneous as well as a composite beam. In this beam, the material properties can be distributed according to the requirement. The axially graded beam overcomes the limitation of conventional composite. Finally, in a double beam, two identical nanobeams are connected by a Winkler-type spring layer. Double beams are used for absorbing the vibration. It reduces deflection and vibration. The double beam is modeled by the coupled differential governing equations.

Some adverse effects such as cracks and the influence of the temperature are considered. Cracks are common defects in nanostructures. Single and multiple cracks are considered in this analysis. According to the model, the crack is replaced by a rotational spring where the crack divides the beam into two segments that are connected to each other by the spring at the crack position. Cracks reduce the overall stiffness of the beam. The effect of temperature is significant for the vibration of nanobeams. The thermal load is compatible with the mechanical load where the thermal load is modeled as an axial load. It reduces the natural frequency.

The main objective of this research is to find suitable techniques for a reliable, cost-effective design that is able to fulfill the desired requirements. That is why the important feature of this research is to apply numerical techniques for solving these problems. Three different approximation techniques such as homotopy perturbation technique, power series method, and Maclaurin series method are used for solving these problems. These techniques are useful for solving linear and non-linear differential equations. However, these techniques are rare to analyze the nano-material. These techniques are applied effectively to scrutinize the model of nanobeams. Obtained results are verified with the results of other researchers in the existing literature. This analysis can be used to design nano-electromechanical devices effectively.

SUMMARY IN ESTONIAN

Nanotalade võnkumise numbriline analüüs

Käesolevas väitekirjas uuritakse nanomaterjalist valmistatud talade omavõnkumisi mitmesuguste kinnitusviiside korral. Väitekirjas on välja töötatud meetodid nanotalade omavõnkesageduse määramiseks astmelise nanotala jaoks erinevate kinnitustingimuste korral; kusjuures astmete nurkades asuvad stabiilsed praod või prao-tüüpi defektid. Prao mõju võnkesagedusele modelleeritakse nn kaalutu väändevedru meetodil. Selle meetodi kohaselt tuleb reaalne astmega tala asendada kahest elemendist koosneva süsteemiga, kus elemendid on omavahel ühendatud väändevedruga, mille jäikus on pöördvõrdeline pinge intensiivsuse koefitsiendiga prao tipu juures. Kuna pinge intensiivsuse koefitsiendi väärtused on leitavad kataloogidest, siis see meetod võimaldab omavahel siduda nanotala omavõnkesageduse ning prao pikkuse ja laiuse.

Väitekiri koosneb sissejuhatusest, viiest peatükist ning kirjanduse loetelust, mis sisaldab 82 nimetust. Sissejuhatus kujutab endast esimest peatükki. Teises peatükis on toodud põhivõrrandid ning põhieeldused. Esimesed kaks peatükki on referatiivsed, ülejäänutes esitatakse originaalseid tulemusi. Kolmandas peatükis esitatakse nanotalade võnkumise võrrandid, mis arvestavad tala elementide pöördinertsit. Need on Euler-Bernoulli võrrandite üldistuseks juhule, kui pöördinertsit arvestamine on kohustuslik. See süsteem on lahendatav ka muutujate eraldamise teel. Neljandas peatükis lahendatakse põhivõrrandite süsteem numbriliselt. Näidatakse muuhulgas, et süsteemi saab hõlpsasti lahendada Maclaurini rea abil. Viies peatükk on pühendatud nanotalade võnkumise uurimisele juhul, kui nanotala on kinnitatud elastsete tugede abil st. toed ei ole jäigad.

Kuuendas peatükis uuritakse pragudega nanotalade võnkumisi arvestades termilisi mõjutusi st. temperatuuripingeid. Väitekirjas saadud tulemusi on võrreldud erijuhtudel kirjandusest leitavate tulemustega ning veendutud, et väitekirjas esitatud tulemused on heas kooskõlas teiste uurijate poolt saadud tulemustega. Väitekirjas saadud tulemuste põhjal on avaldatud koos juhendajaga 10 teadusartiklit.

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PUBLICATIONS

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3. 46th International JVE Conference, 2020 in St. Petersburg, Russia, June 29 - July 1 2020.

4. Biosystems Engineering 2021, University of life science, Tartu, Estonia, 5-7 May, 2021.

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Teadustegevus

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